-	ABBREVIA	TIONS			DES	GN CRITERIA	·		·	·
М	ACI AISC ADDL ADJ.	AMERICAN CONCRETE INSTITUTE AMERICAN INSTITUTE OF STEEL CONSTRUCTION ADDITIONAL ADJACENT	M. MAT. MAX. M.C. MECH.	MOMENT MATERIAL MAXIMUM MOMENT CONNECTION MECHANICAL	1.0	 CODES AND SPECIFICA FLORIDA BUILDING ACI 318-14 BUILDING AISC 360-16 SPECIF ASCE 7-16 MINIMUM 	CODE 2020. S CODE REQUICATION FOR	STRUCTURA	AL STEEL BUILDI	NGS
•	AGG. ALT. APPROX. ARCH. &	AGGREGATE ALTERNATE APPROXIMATELY ARCHITECT AND	MEZZ. MID. MIN. MISC. MFR.	MEZZANINE MIDDLE MINIMUM MISCELLANEOUS MANUFACTURER	2.0 2.1	DESIGN LOADS DEAD LOADS:				
L	@ B. TO B. BEV.	AT BACK TO BACK BEVEL	MTL. N.I.C. NO. OR #	METAL NOT IN CONTRACT NUMBER		SELF WEIGHT OF STRUCONTRACT DRAWINGS FABRICATED WITH RECTHE FOLLOWING SUST	S, WITH ALL P GULAR WEIG	RECAST COI HT CONCRET	NCRETE COMPO	NENTS
•	B.F. B.L. BLDG. B/ OR BOT		NOM. N.S. N.T.S.	NOMINAL NEAR SIDE NOT TO SCALE	2.1.1	PARKING STRUCTURE A. FLOOR/ROOF SUPE INCLUDING PRECASE	RIMPOSED LO	OADING (SUS	SPENDED, MISC.)	3 PSF
К	BRDG. BTWN BRG C C.G.	BRIDGING BETWEEN BEARING CHANNEL CENTER OF GRAVITY	O.C. O.D. O.F. O.H. OPN. OPP.	ON CENTER OUTSIDE DIAMETER OUTSIDE FACE OPPOSITE HAND OPENING OPPOSITE		B. EXTERIOR CLADDIN C. STEEL FRAMED CAN STRUCT. STEEL FRA 2" 20 GA STEEL FOR	G (PER VERT NOPIES: AMING		OT TO EXCEED	20 PSF /GT PER AISC
•	C.I.P. C.J.	CAST IN PLACE CONSTRUCTION JOINT OR CONTROL JOINT CENTERLINE	OTTG. PAR. PART.	OUTSTANDING PARALLEL PARTITION	2.1.2	2" REG. WGT CONC. ROOFING MISCELLANEOUS (E				2 PSF 2 PSF
J	CLR. C.M.U. COL. COMP. CONC.	CLEAR CONCRETE MASONRY UNIT COLUMN COMPRESSION CONCRETE	P.C.C. P.C.F. P.C.I. PERP. PL	PRE-CAST CONCRETE POUNDS PER CUBIC FOOT POUNDS PER CUBIC INCH PERPENDICULAR PLATE		A. GLAZING, PER UNIT NOT TO EXCEED B. ENCLOSURE ROOF ROOFING, INSULATI	SUSPENDED	(MECHANICA	 AL, ELECTRICAL),	15 PSF
•	CONN. CONST. CONT. COV. PL.	CONNECTION CONSTRUCTION CONTINUOUS COVER PLATE	P.S.F. P.S.I. PT.	POUNDS PER SQUARE FOOT POUNDS PER SQUARE INCH POINT		LIVE LOAD (OCCUPANO PARKING STRUCTURE	CY LOADS, U.	N.O.):		
Н	DBL. DEG. DET. DIAG.	DOUBLE DEGREE DETAIL DIAGONAL	RAD. REINF. REQ. REQ'D	RADIUS REINFORCEMENT REQUIRE REQUIRED		A. TYPICAL PARKING L HEREIN BELOW 1. PARKING STALLS 2. MAXIMUM CONCE 3. PROVISIONS UND	S, DRIVE ISLE ENTRATED W	S HEEL LOAD (OVER AREA OF 4	40 PSF .5"x4.5"3,000 LB
•	DIAM. DIM. D.L. D.P. DWG.	DIAMETER DIMENSION DEAD LOAD DRILLED PIER DRAWING	SCHED. SECT. S.F. SHT. SIM.	SCHEDULE SECTION SQUARE FOOT (FEET) SHEET SIMILAR		PASSENGER VEH	HICLE OR A LI DF 8,500 LB A E TO ACCOM	GHT-DUTY T S DEFINED E MODATE HEA	RUCK WITH A MA BY THE U.S. E.P.A AVIER VEHICLES	XIMUM GWVR NO PROVISIONS
G	DWLS. EA. E.F. E.J.	EACH EACH FACE EXPANSION JOINT	SPEC. SPEC'D STL. STD. STIFF.	SPECIFICATION SPECIFIED STEEL STANDARD STIFFENER	2.2.2	STAIR AND ELEVATOR A. OCCUPANCY B. ELEVATOR MACHINI			PEF	100 PSF R CONTRACTOR'S
•	EL. ELEC. ELEV.	ELEVATION ELECTRICAL ELEVATOR	STIR. STRUCT. SUB-CONT SUPT.	STIRRUP STRUCTURE SUB-CONTRACTOR SUPPORT	2.3	C. ELEVATOR SERVICE				
F	F. TO F. FABR. FIN. FIN. FL. FL.	FACE TO FACE FABRICATOR FINISH FINSHED FLOOR FLOOR	SYM. T/ T&B TEMP.	SYMMETRICAL TOP OF TOP AND BOTTOM TEMPERATURE		GROUND SNOW LOA MINIMUM ROOF LIVE LOA				· ·
•	FNDN. FP F.S. FT.	FOUNDATION FIRE PROTECTION FAR SIDE FOOT OR FEET	TEN. THK. TYP. U.N.	TENSION THICK TYPICAL UNLESS NOTED	2.5	RAIN LOADS (APPLICABENCLOSURES ONLY) RESPECTIVE STRUCTU CODE 2020 SECTION 16	SHAL JRAL MEMBEI	L BE INCLUE RS IN ACCOF	DED IN THE DESIGNATION OF THE PROPERTY OF THE	ON OF THE ORIDA BUILDING
Е	FTG. GA. GALV. G.C.	FOOTING GAUGE GALVANIZED GENERAL CONTRACTOR	U.N.O. V VERT.	UNLESS NOTED UNLESS NOTED OTHERWISE SHEAR VERTICAL	0.0	MANUFACTURER'S ENC PROJECT PLUMBING A SHALL BE ASSUMED A	ND/OR CIVIL	ENGINEER. 1	00-YEAR HOURL	Y RAINFALL
•	GR. GR. BM. G.S. H.B.	GRADE GRADE BEAM GALVANIZED STEEL HOOKED BAR	W/ W/O WF W XxXX	WITH WITHOUT WIDE FLANGE WIDE FLANGE SECTION	2.6	WIND LOADS A. ALL WIND LOADS SH CODE 2020 SECTION FOLLOWS: • RISK CATEGORY	N 1609 AND A	SCE 7 CHAP	TERS 26 THROUG	SH 30 AS II
D	HD. HK. HORIZ. H.P.	HEADED HOOK HORIZONTAL HIGH POINT	W.L. W.P. WPRF. W.S. W.W.F.	WIND LOAD WORK POINT WATER PROOFING WATER STOP WELDED WIRE FABRIC		 ULTIMATE DESIG NOMINAL DESIGN WIND EXPOSURE WIND INTERNAL CALCULATED WII 	N WIND SPEE : PRESSURE C	D V asd COEFFICIENT		131 MPH D
•	H.S. H.S.B. HSS HGT.	HEADED STUD HIGH STRENGTH BOLT HOLLOW STRUCTURAL SECTION (TUBE) HEIGHT	VV.VV.F.	WELDED WIRE FABRIC		HGT ABOVE MA	AIN WIND FOR	RCE C	OMPONENTS AN AWAY FROM CORNERS	D CLADDING AT CORNERS
C	I.D. I.F. INFO. INT. INTERM.	INSIDE DIAMETER INSIDE FACE INFORMATION INTERIOR INTERMEDIATE				15 - 20 20 - 25 25 - 30	N-S 76 PSF 78 PSF 80 PSF 82 PSF 84 PSF	E-W 63 PSF 66 PSF 67 PSF 69 PSF 71 PSF	97 PSS204 97 PSF 97 PSF 97 PSF 97 PSF	120 PSF 120 PSF 120 PSF 120 PSF 120 PSF
	JST. JT.	JOIST JOINT				FOR DESIGN WIND I ON DWG. XXX AND			•	NLARGED PLANS
B •	K. K.L.F. K.S.F L LB.	KIPS KIPS PER LINEAL FOOT KIPS PER SQUARE FOOT ANGLE POUND				NOTE: WIND PRESS BEEN BASED ON EF ACTUAL EFFECTIVE CALCULATION OF TI CONTRACTOR SHAL CALCULATION TO TI	FECTIVE WIN WIND AREA HE ABOVE PF LL RE-CALCUI	ID AREA OF T IS GREATER RESSURES, T LATE THE PR	10 FT ² OR LESS. THAN THE BASIS THE CLADDING SI ESSURES AND S	N THE EVEN THE SUSED FOR PECIALTY
	L.L. L.L.H. L.L.V.	LIVE LOAD LONG LEG HORIZONTAL LONG LEG VERTICAL			2.7	SEISMIC LOADSPER FLORIDA BUILDING				
Α	LONG. L.P. LTWT.	LONGITUDINAL LOW POINT LIGHT WEIGHT			2.8	FUTURE EXPANSION: N IN ANY DIRECTION.	IO PROVISIO	NS HAVE BEI	EN MADE FOR FU	ITURE EXPANSION

GENERAL NOTES

- ALL NOTES GIVEN ON THESE DRAWINGS ARE SUPPLEMENTAL TO THE PROJECT SPECIFICATIONS AND ARE NOT INTENDED TO REPLACE THEM. IN THE EVENT OF AN APPARENT CONFLICT BETWEEN THE NOTES AND THE PROJECT SPECIFICATIONS, THE CONTRACTOR MUST OBTAIN CLARIFICATION IN WRITING FROM THE ARCHITECT.
- 2. ALL DESIGN AND CONSTRUCTION SHALL BE DONE IN ACCORDANCE WITH THE MOST STRINGENT OF THE GOVERNING CODES IF NOT SPECIFICALLY INDICATED. THE LATEST EDITION OF THE CITY, STATE AND LOCAL CODES AND ALL OTHER PERTINENT CODES, REGULATIONS AND ORDINANCES MUST BE USED.
- 3. DRAWINGS ARE NOT TO BE USED FOR SHOP DETAILING OR FOR CONSTRUCTION UNLESS SPECIFICALLY INDICATED BY THE ARCHITECT/ENGINEER "FOR DETAILING" OR "FOR CONSTRUCTION". THESE DRAWINGS ARE NOT TO BE REPRODUCED FOR THE PURPOSE OF USING THEM AS SHOP DETAIL DRAWINGS.
- 4. STRUCTURAL DRAWINGS ARE INTENDED TO BE USED TOGETHER WITH CIVIL ARCHITECTURAL, MECHANICAL AND ELECTRICAL DRAWINGS. THE CONTRACTOR IS RESPONSIBLE FOR COORDINATING THE REQUIREMENTS OF ALL DRAWINGS IN TO THEIR SHOP DRAWINGS AND WORK.
- 5. PROVIDE SLEEVE/OPENING LAYOUTS FOR ALL PIPES, CONDUITS AND SIMILAR FACILITIES THROUGH STRUCTURAL MEMBERS (ALL TRADES INCLUDED) TO THE ARCHITECT FOR APPROVAL PRIOR TO DETAILING OF CAST-IN-PLACE COMPONENTS OR FABRICATION OF FRAMING MEMBERS.
- 6. NO OPENING OTHER THAN THOSE SHOWN ON DESIGN DRAWINGS AND APPROVED SHOP DRAWINGS, SHALL BE MADE WITHOUT THE WRITTEN APPROVAL OF THE ARCHITECT.
- 7. SMALL OPENINGS (NOT EXCEEDING 18 INCHES IN DIAMETER OR LARGEST DIMENSION FOR RECTANGULAR CONFIGURATIONS) ARE GENERALLY NOT SHOWN ON THE STRUCTURAL DRAWINGS. REFER TO ARCHITECTURAL, MECHANICAL. PLUMBING, FIRE PROTECTION AND ELECTRICAL DRAWINGS FOR LOCATIONS AND DIMENSIONS OF THOSE OPENINGS. PROVIDE REINFORCEMENT AROUND OPENINGS PER TYPICAL DETAILS SHOWN ON STRUCTURAL DRAWINGS. MULTIPLE OPENINGS SPACED CLOSER TOGETHER THAN THE DIAMETER (OR SMALLER DIMENSION FOR RECTANGULAR CONFIGURATION) OF THE LARGER OF ADJACENT OPENINGS OR 6 INCHES, WHICHEVER IS GREATER. SHALL BE TREATED AS ONE "EFFECTIVE" OPENING DEFINED BY OUTER TANGENT LINES CONNECTING ADJACENT OPENINGS IN A CLUSTER.
- 8. ACTUAL OR "EFFECTIVE" OPENING INTRODUCED AS A RESULT OF SLEEVE/OPENING LAYOUT/PLACEMENT BY THE CONTRACTOR, OF SIZE LARGER THAN THAT SHOWN IN TYPICAL DETAIL FOR REINFORCEMENT AROUND OPENINGS, SHALL BE RE-ENGINEERED AT CONTRACTOR'S OWN EXPENSE.
- 9. NO CHANGE TO ANY STRUCTURAL COMPONENT (SIZE, DIMENSIONS, MATERIALS, ETC.), SHALL BE MADE WITHOUT THE WRITTEN APPROVAL OF THE ARCHITECT. THIS INCLUDES, BUT IS NOT LIMITED TO REVISIONS DUE TO MISLOCATION, MISFIT, OR ANY OTHER CONSTRUCTION ERRORS.
- 10. SUPPORT ALL ROOF MOUNTED OR SUSPENDED EQUIPMENT, ONLY ON/FROM JOISTS, TRUSSES, BEAMS, OR OTHER STRUCTURAL MEANS SPECIFICALLY DESIGNATED FOR SUCH PURPOSE. IF NO SUPPORT HAS BEEN DESIGNATED. OR IF QUESTION ARISES, NOTIFY THE ARCHITECT PRIOR TO ERECTION OF EQUIPMENT
- 11. DO NOT SCALE THESE DRAWINGS. USE DIMENSIONS SHOWN ON THE DRAWINGS.
- 12. THE CONTRACTOR SHALL SUPERVISE AND DIRECT ALL WORK SO AS TO MAINTAIN SOLE RESPONSIBILITY FOR ALL CONSTRUCTION MEANS, METHODS, TECHNIQUES, SEQUENCES AND PROCEDURES.
- 13. THE CONTRACTOR SHALL BE RESPONSIBLE FOR PROPER DESIGN, ADEQUATE INSTALLATION AND MAINTENANCE OF ANY AND ALL SHEETING, SHORING AND UNDERPINNING AGAINST EXISTING STRUCTURES AS REQUIRED, SO THAT THEY ARE NOT ENDANGERED BY THIS CONSTRUCTION. TEMPORARY MEMBERS AND CONNECTIONS SHALL NOT BE REMOVED UNTIL PERMANENT MEMBERS ARE IN PLACE AND FINAL CONNECTIONS ARE MADE APPROVED.
- 14. THE CONTRACTOR SHALL PROVIDE ALL THE NECESSARY MEASURES AND PRECAUTIONS TO PREVENT DAMAGE AND SETTLEMENT OF EXISTING OR NEW CONSTRUCTION INSIDE OR OUTSIDE THE PROJECT LIMITS DURING EXCAVATION. ANY DAMAGE TO NEW OR EXISTING CONSTRUCTION INSIDE OR OUTSIDE OF THE PROJECT LIMITS, CAUSED BY CONSTRUCTION TECHNIQUES SHALL BE THE RESPONSIBILITY OF THE CONTRACTOR.
- 15. THE CONTRACTOR SHALL ENGAGE PROPERLY QUALIFIED PERSONS TO DETERMINE WHERE AND HOW TEMPORARY PRECAUTIONARY MEASURES SHALL BE USED, AND INSPECT THE SAME IN THE FIELD.
- 16. THE STRUCTURE IS DESIGNED TO FUNCTION AS A UNIT UPON COMPLETION. BASED ON THE CONTRACTOR'S CONSTRUCTION METHODS AND SEQUENCING OF CONSTRUCTION, THE CONTRACTOR SHALL RETAIN A PROFESSIONAL ENGINEER CURRENTLY LICENSED TO PRACTICE IN THE STATE WHERE THE WORK IS LOCATED. TO DESIGN LATERAL SUPPORT SYSTEM REQUIRED TO RESIST LATERAL LOADS AND FOR OVERALL STABILITY OF THE STRUCTURE UNTIL COMPLETION. THE CONTRACTOR SHALL FURNISH AND INSTALL ALL THE NECESSARY BRACING AND SUPPORTS AND SHALL BE RESPONSIBLE FOR THE OVERALL STABILITY OF THE STRUCTURE DURING CONSTRUCTION.
- 17. THE CONTRACTOR IS RESPONSIBLE FOR LIMITING THE AMOUNT OF CONSTRUCTION LOAD IMPOSED UPON STRUCTURAL FRAMING. CONSTRUCTION LOAD SHALL NOT EXCEED THE CAPACITY OF THE FRAMING AT THE TIME THE LOADS ARE IMPOSED.
- 18. THE CONTRACTOR'S CONSTRUCTION AND/OR ERECTION SEQUENCES SHALL RECOGNIZE AND CONSIDER THE EFFECTS OF THERMAL MOVEMENTS OF THE STRUCTURAL ELEMENTS.
- 19. THE CONTRACTOR SHALL VERIFY IN FIELD ALL DIMENSIONS, ELEVATIONS, MEMBER LOCATIONS AND MEMBER SIZES AS SHOWN ON THE CONTRACT DRAWINGS FOR THE EXISTING CONSTRUCTION, IF ANY. CONTRACTOR SHALL NOTIFY THE ARCHITECT OF ANY DISCREPANCIES IN THE FORM OF DETAILED SKETCHES AT LEAST FOUR (4) WEEKS PRIOR TO THE SCHEDULED START OF ANY STRUCTURAL DEMOLITION, OR FABRICATION, OR INSTALLATION AFFECTED BY SUCH DISCREPANCIES. FOR DISCREPANCIES BETWEEN THE EXISTING CONSTRUCTION AND THE CONTRACT DRAWINGS, THE CONTRACTOR SHALL OBTAIN DIRECTION FROM THE ARCHITECT BEFORE PROCEEDING.

GENERAL NOTES (CONT.)

- 20. NEITHER THE ARCHITECT NOR THE STRUCTURAL ENGINEER SHALL BE RESPONSIBLE FOR OR HAVE CONTROL OR CHARGE OF CONSTRUCTION MEANS. METHODS, TECHNIQUES, SEQUENCES OR PROCEDURES FOR THE SAFETY PRECAUTIONS AND PROGRAMS IN CONNECTION WITH THIS PROJECT. NEITHER THE ARCHITECT NOR THE STRUCTURAL ENGINEER SHALL BE RESPONSIBLE FOR CONTRACTOR'S FAILURE TO CARRY OUT HIS/HER WORK IN ACCORDANCE WITH THE CONTRACT DOCUMENTS. NEITHER THE ARCHITECT NOR THE STRUCTURAL ENGINEER SHALL BE RESPONSIBLE FOR, OR HAVE CONTROL OVER, THE ACTS OF OMISSIONS BY THE CONTRACTOR, SUBCONTRACTOR, ANY OF THEIR AGENTS, EMPLOYEES, OR ANY OTHER PERSONS PERFORMING ANY OF THE WORK, OR FOR THE FAILURE OF ANY OF THEM TO CARRY OUT THE WORK IN ACCORDANCE WITH THE CONTRACT DOCUMENTS.
- 21. MINIMUM VERTICAL CLEARANCE IN THE PARKING STRUCTURE SHALL BE AS
- FOLLOWS: A. LEVELS GRADE TO SECOND & SECOND TO THIRD == 8'-3"
- B. LEVELS THIRD TO ROOF == 7'-11" THE CONTRACTOR SHALL ASCERTAIN THAT THE REQUIRED CLEAR HEIGHTS

LISTED ABOVE, WILL BE PROVIDED PRIOR TO FABRICATION. THE CONTRACTOR SHALL NOTIFY THE ARCHITECT IN WRITING IMMEDIATELY IF THE REQUIRED CLEAR HEIGHT IS NOT ATTAINABLE. THE ARCHITECT WILL PROVIDE THE DIRECTION UPON RECEIVING THIS NOTIFICATION.

GENERAL FOUNDATION NOTES

- 1. THE FOUNDATION DESIGN IS BASED ON THE GEOTECHNICAL ENGINEERING REPORT DATED PROJECT No. . IT IS THE RESPONSIBILITY OF THE CONTRACTOR TO REVIEW THE GEOTECHNICAL ENGINEERING REPORT PRIOR TO BIDDING. ALL WORK SHALL BE DONE PER THE RECOMMENDATIONS GIVEN IN THE GEOTECHNICAL REPORT.
- 2. FOR EARTHWORK AND SITE PREPARATION REFER TO THE PROJECT SPECIFICATIONS AND THE ABOVE GEOTECHNICAL REPORT. AREAS OF LOOSE, WET, ORGANIC OR OTHERWISE UNSUITABLE OR UNSTABLE MATERIAL SHALL BE LOCALLY OVER-EXCAVATED AND REPLACED WITH ENGINEERED FILL ACCEPTABLE TO THE OWNER'S GEOTECHNICAL ENGINEER. UNDERCUTS SHALL EXTEND LATERALLY ON A 2(H):1(V) SLOPE, AT LEAST 1 (ONE) FT AWAY FROM THE EDGE OF ANY FOOTING. ALL SITE PREPARATION WORK WITHOUT EXCEPTIONS SHALL BE PERFORMED UNDER THE DIRECTION OF THE OWNER'S GEOTECHNICAL ENGINEER.
- 3. PARKING STRUCTURE COLUMNS, BEARING WALLS AND STAIR/ELEVATOR FOUNDATION PADS OR MATS SHALL BE SUPPORTED ON .
- 4. ALL SHALLOW FOUNDATIONS OTHER THAN THOSE DESCRIBED UNDER ITEMS (2) THROUGH (6) HEREIN ABOVE, SHALL BEAR UPON UNDISTURBED NATURAL SUBGRADE OF MEDIUM FINE TO SILTY FINE SAND, OR TESTED AND APPROVED ENGINEERED FILL. THE REQUIRED MINIM SOIL BEARING CAPACITY OF THESE SUBGRADES SHALL BE NOT LESS THAN 2,000 PSF AS VERIFIED IN SITU BY THE OWNER'S GEOTECHNICAL ENGINEER.
- 5. MATERIALS USED FOR ENGINEERED FILL SHALL CONFORM TO THE REQUIREMENTS OF THE GEOTECHNICAL REPORT AND BE ENVIRONMENTALLY CLEAN, FREE OF OF ORGANIC AMTTER AND OTHER DELETERIOUS SUBSTANCES AND ACCEPTABLE TO THE OWNER'S GEOTECHNICAL ENGINEER.
- 6. ALL ENGINEERED FILL MATERIAL SHALL BE PLACED IN INDIVIDUAL LIFTS OF LOOSE THICKNESS NOT EXCEEDING 9 (NINE) INCHES. AND COMPACTED TO AT LEAST 95% OF THE MATERIAL'S MAXIMUM MODIFIED PROCTOR DRY DENSITY PER ASTM D1557 MOISTURE CONTENT SHOULD BE WITH (-)1% TO (+)3% OF THE OPTIMUM VALUE PRIOR TO PLACEMENT AND COMPACTION.
- 7. ENCOUNTERED OPERATING UTILITIES SHALL BE TREATED IN ACCORDANCE WITH THE REQUIREMENTS OF LOCAL CODES, REGULATIONS AND ORDINANCES TO THE SATISFACTION OF AUTHORITIES HAVING JURISDICTION. IT SHALL BE THE CONTRACTOR'S RESPONSIBILITY TO OBTAIN AND/OR VERIFY INFORMATION CONCERNING SUCH UTILITIES PRIOR TO COMMENCEMENT OF THE WORK. FAILURE TO DO SO SHALL NOT BE CONSIDERED A CAUSE FOR CONSIDERATION OF SUCH ENCOUNTERS AS UNFORESEEN CONDITIONS.
- 8. THE CONTRACTOR SHALL NOTIFY THE ARCHITECT IN THE EVENT ANY EXISTING UTILITIES, UTILITY STRUCTURES OR ANY OBSTRUCTION INTERFERES WITH THE PROPER INSTALLATION OF THE FOUNDATION WORK. THE ARCHITECT WILL REVIEW THE SITUATION AND ISSUE AN APPROPRIATE DIRECTION.
- 9. ALL SOIL SUBGRADES FOR SHALLOW FOUNDATIONS AND SLABS-ON-GRADE SHALL BE INSPECTED AND APPROVED BY THE OWNER'S GEOTECHNICAL ENGINEER PRIOR TO PLACING CONCRETE.
- 10. FOR DEEP FOUNDATION NOTES SEE "DRILLED PIER FOUNDATION NOTES" ON THIS DRAWING. DRILLED PIER CAPS AND GRADE BEAMS SPANNING BETWEEN AND SUPPORTED BY THE DRILLED PIERS SHALL NOT BE CONSIDERED SOIL SUPPORTED FOUNDATIONS. HOWEVER, DO NOT PLACE CONCRETE OF THESE STRUCTURES ON ANY SUBGRADE WITH LESS THAN 1,500 PSF IN LOAD CARRYING CAPACITY. SUBGRADES PREPARED FOR THESE ELEMENTS MUST BE ADEQUATE TO SUPPORT THEM UNTIL SUCH MEMBERS ARE FULLY CURED. THE CONTRACTOR SHALL TAKE WHATEVER MEANS NECESSARY TO PROVIDE THESE CONDITIONS.
- 11. ALL SHALLOW FOUNDATIONS SHALL BEAR A MINIMUM OF 2'-0" BELOW THE LOWEST OF ADJACENT GRADES INSIDE OR OUTSIDE THE LIMITS OF THE STRUCTURE FOOTPRINT.
- 12. THE CONTRACTOR MUST PROVIDE SURFACE DRAINAGE AND MEANS TO REMOVE WATER TO PROTECT ALL EXCAVATIONS FROM FLOODING. FLOODING OF ANY EXCAVATION AFTER APPROVAL OF THE SUBGRADE BY THE OWNER'S GEOTECHNICAL ENGINEER WILL BE CAUSE FOR COMPLETE RE-PREPARATION AND RE-APPROVAL OF THE AFFECTED SUBGRADE.
- 13. THE CONTRACTOR SHALL PROVIDE ALL NECESSARY MEASURES TO PREVENT ANY WATER FROM ENTERING ANY FOOTING OR SLAB SUBGRADE BEFORE AND AFTER PLACEMENT OF CONCRETE AND UNTIL SUCH SUBGRADES ARE FULLY PROTECTED BY THE PERMANENT BUILDING STRUCTURE. SHOULD WATER ENTER A FOOTING EXCAVATION AFTER THE SUBGRADE APPROVAL. SUCH SUBGRADE SHALL BE RE-INSPECTED AND RE-APPROVED BY THE OWNER'S GEOTECHNICAL ENGINEER FOLLOWING REMOVAL OF WATER. THIS REQUIREMENT APPLIES IN EQUAL MEASURE TO MUD SLABS IF USED.



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1st Avenue S. **Public Parking** Garage

Owner

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Schematic Design

Date

Project #: 11-22107.00-1 Project Issued: Sheet Issued: 01/05/22

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Revision

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Sheet Title: Abbreviations, Design Criteria and **General Notes**

S001

Sheet No.

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GENERAL FOUNDATION NOTES (CONT.)

- 14. THE SOIL AND FILL SUBGRADE FOR SLABS-ON-GRADE SHALL BE INSPECTED AND APPROVED BY THE OWNER'S GEOTECHNICAL ENGINEER PRIOR TO PLACING "SLIP" SHEET
- 15. NO FOOTINGS OR SLABS SHALL BE PLACED INTO OR AGAINST SUBGRADE CONTAINING FREE WATER. SUCH ACTION SHALL BE SUBJECT TO REMOVAL OF CONCRETE AND RE-PREPARATION OF THE SUBGRADE PRIOR TO PLACEMENT OF
- ALL FOUNDATION AND/OR RETAINING WALLS SHOWN WITH UNEVEN BACKFILL ON THE SIDES SHALL BE SIMULTANEOUSLY BACKFILLED AND COMPACTED ON BOTH SIDES TO THE LOWEST ADJACENT BACKFILL GRADE. MAXIMUM DIFFERENTIAL ELEVATION IN SUCH SIMULTANEOUS OPERATION SHALL NOT EXCEED 1 (ONE) FOOT.
- 16. THE CONTRACTOR SHALL LOCATE CONSTRUCTION JOINTS IN CONTINUOUS STRIP FOOTINGS SUCH THAT THE MAXIMUM LENGTH OF A CONCRETE PLACEMENT DOES NOT EXCEED 60 FEET. PLACEMENT OF FOOTINGS SHALL FOLLOW A CHECKER-BOARD PATTERN WITH THE TIME LAG BETWEEN ADJACENT PLACEMENTS AS LONG AS POSSIBLE BUT NOT LESS THAN 72 HOURS. CONSTRUCTION JOINTS IN WALLS, IF NOT SPECIFICALLY SHOWN ON THE DRAWINGS, SHALL BE COORDINATED WITH THE ARCHITECTURAL DESIGN INTENT AND LOCATED TO MINIMIZE THE EFFECT OF SHRINKAGE AND TEMPERATURE CHANGES. LENGTH OF WALL SEGMENT PLACED AT ANY ONE TIME IS NOT LIMITED. THE CONTRACTOR SHALL SUBMIT A PROPOSED JOINT LAYOUT AND CONCRETE PLACEMENT SEQUENCE TO THE ARCHITECT FOR
- 17. THE CONCRETE FOR EACH ISOLATED MAT OR SPREAD FOOTING SHALL BE PLACED IN ONE (1) CONTINUOUS PLACEMENT, UNLESS SPECIFICALLY INDICATED OTHERWISE ON THE CONTRACT DRAWINGS.
- 1. CANTILEVER RETAINING WALLS SCHEDULED AND SHOWN ON THE DRAWINGS HAVE BEEN DESIGNED TO NOT REQUIRE LOW SIDE BRACING, AND MAY BE BACKFILLED TO A MAXIMUM OF 75% OF THE WALL HEIGHT FFROM TOP OF FIOOTING UPON WALL CONCRETE ATTAINING A MINIMUM OF 75% OF THE SPECIFIED COMPRESSIVE STRENGTH.
- SEE PLUMBING DRAWINGS FOR TRENCH DETAILS AND SPECIAL GRANULAR FILL MATERIALS. REFER TO CIVIL DRAWINGS FOR LAYOUT AND DETAILS OF UTILITY SUPPORTS. SEE ARCHITECTURAL DRAWINGS FOR ALL WATERPROOFING AND DAMPPROOFING DETAILS.
- 3. UNLESS NOTED OTHERWISE, ALL ELEVATIONS ARE REFERENCED TO THE NORTH AMERICAN VERTICAL DATUM N.A.V.D. 1988.
- 4. THE CONTRACTOR SHALL EXERCISE DUE CARE AND CAUTION WORKING IN THE AREAS ADJOINING EXISTING CONSTRUCTION TO REMAIN. THE CONTRACTOR SHALL BE FULLY RESPONSIBLE FOR PROVIDING AND MAINTAINING MEASURES PROTECTING EXISTING CONSTRUCTION. ANY AND ALL DAMAGES TO THE EXISTING CONSTRUCTION CAUSED BY THE CONTRACTOR'S MEANS AND METHODS AND/OR CONTRACTOR'S FAILURE TO PROVIDE PROTECTION SHALL BE REPAIRED BY THE CONTRACTOR TO THE SATISFACTION OF THE AUTHORITY HAVING JURISDICTION AND AT NO COST TO THE GOVERNMENT.
- 5. SEE SPECIFICATIONS FOR ADDITIONAL REQUIREMENTS.

CONCRETE NOTES

- 1. ALL CONCRETE WORK SHALL CONFORM TO THE REQUIREMENTS OF ACI 301. THIS DOCUMENT SHALL BE AVAILABLE IN THE FIELD OFFICE.
- 2. EXCEPT WHERE OTHERWISE INDICATED, CONCRETE TYPES AND MINIMUM 28-DAY COMPRESSIVE STRENGTHS SHALL BE AS FOLLOWS:

CO	MPRESSIVE STRENGTHS SHALL BE AS FOLLOW	/ S:
Α	WALL AND COLUMN FOOTINGS, ELEVATOR PIT MATS, U.N.O.	4,500 PSI REGULAR WEIGHT
В	RETAINING WALLS, FOUNDATION WALLS, U.N.O.	4,500 PSI REGULAR WEIGHT
С	SLAB-ON-GRADE	4,000 PSI REGULAR WEIGHT
D	PRECAST CONCRETE COMPONENTS	SEE PRECAST CONC. NOTES, THIS DWG.
Е	TOPPING AND WASHES OVER PRECAST CONCRETE MEMBERS	5,000 PSI REGULAR WEIGHT
F	ALL OTHER	4,000 PSI REGULAR WEIGHT

- 3. SEE PROJECT SPECIFICATIONS FOR CONCRETE MIXTURES REQUIREMENTS.
- 4. CEMENT SHALL CONFORM TO ASTM C150 TYPE I OR TYPE II. USE ONLY ONE BRAND OF CEMENT FOR ALL EXPOSED TO VIEW CONCRETE. AGGREGATES SHALL CONFORM TO ASTM C33 (REGULAR WEIGHT). ALL CONCRETE SHALL CONTAIN AN APPROVED WATER REDUCING ADMIXTURE. NO CALCIUM CHLORIDE SHALL BE USED IN ANY CONCRETE. SEE PROJECT SPECIFICATIONS FOR ADDITIONAL INFORMATION.
- 5. ALL REINFORCING BARS SHALL CONFORM TO ASTM A615 OR ASTM A706, GRADE 60 UNLESS OTHERWISE NOTED ON THE DRAWINGS. WHERE WELDING OF REINFORCEMENT BARS IS REQUIRED, USE STEEL BARS CONFORMING TO ASTM A706 GRADE 60, UNLESS NOTED OTHERWISE. ALL WELDED WIRE FABRIC SHALL CONFORM TO ASTM A185. REFER TO THE PROJECT SPECIFICATIONS FOR REQUIREMENTS RELATIVE TO COATED REINFORCEMENT.
- 6. ALL MILD STEEL REINFORCEMENT SHALL BE DETAILED, FABRICATED, LABELED, SUPPORTED AND SPACED IN FORMS AND SECURED IN PLACE IN ACCORDANCE WITH THE ACI DETAILING MANUAL SP-066, LATEST EDITION. BAR SUPPORTS IN CONTACT WITH EXPOSED SURFACES SHALL BE PLASTIC TIPPED.
- 7. ALL 135° HOOKS SHALL BE MINIMUM 6d, WHERE "d" IS BAR DIAMETER. ALL OTHER HOOKS SHALL BE STANDARD ACI 90° OR 180° HOOKS. SEE DETAIL 1 THIS DWG. AND ACCOMPANYING TABLE FOR HOOKED BAR DEVELOPMENT REQUIREMENTS.

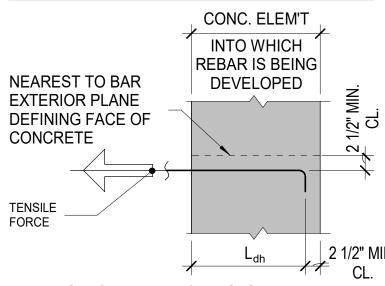
CONCRETE NOTES (CONT.)

- 8. MINIMUM CLEAR SPACING OF PARALLEL MILD STEEL REINFORCING BARS IN A LAYER SHALL NOT BE LESS THAN A BAR DIAMETER (USE LARGER OF THE TWO ADJACENT BARS FOR DISSIMILAR SIZES), BUT SHALL NOT BE LESS THAN ONE (1) INCH. FOR BUNDLED BARS, USE EQUIVALENT DIAMETER OF A BUNDLE BASED ON A CROSS-SECTIONAL AREA. WHERE PARALLEL BARS ARE TO BE PLACED IN TWO (2) OR MORE LAYERS, PLACE UPPER LAYER BARS DIRECTLY ABOVE THE LOWER LAYER BARS. CLEAR DISTANCE BETWEEN THE ADJACENT LAYERS SHALL NOT BE LESS THAN ONE (1) INCH.
- 9. UNLESS OTHERWISE NOTED, PROVIDE CLEAR CONCRETE COVER TO REINFORCEMENT IN ALL CAST-IN-PLACE CONCRETE AS FOLLOWS, BUT IN NO CASE LESS THAN THE VALUES INDICATED IN ACI 301 ART. 3.3.2.3:

MEMBER TYPE	EXPOSED*	NOT EXPOSED
CAST AGAINST AND PERMANENTLY EXPOSED TO EARTH	3"	
BEAM - TO STIRRUP	1 1/2"	1 1/2"
BEAM - TO PRIMARY REBAR	2"	
COLUMN - TO TIES	2"	1 1/2"
"CRASH" WALL	1 1/2"	
SLAB/TOPPING - TOP	2"	3/4"
SLAB - BOTTOM (INTERIOR)	1"	3/4"
SLAB - BOTTOM (EXTERIOR)	1 1/2"	1"
WALL	2"	1"

- * "EXPOSED" IN THE ABOVE TABLE MEANS EXPOSURE TO MOISTURE IN SERVICE
- 10. ALL SLABS-ON-GRADE SHALL BE REINFORCED WITH STRUCTURAL FIBER OR MILD STEEL REINFORCEMENT AS NOTED ON PLANS. WHERE NEITHER FIBER NOR REINFORCEMENT IS NOTED ON THE DRAWINGS, PROVIDE AT LEAST ONE (1) LAYER OF 6x6-W2xW2 W.W.F., UNLESS NOTED OTHERWISE. PROVIDE ONE (1) LAYER OF 6x6-W1.4xW1.4 W.W.F. CONTINUOUS IN ALL CONCRETE FILLS OVER THE STRUCTURAL SLAB, UNLESS NOTED OTHERWISE. ALL WELDED WIRE FABRIC REGARDLESS OF SLAB COMPONENT SHALL BE PROPERLY SUPPORTED ON APPROPRIATE CHAIRS/BOLSTERS AND TIED DOWN SECURELY. SHEETS SHALL BE LAPPED TO FULL PANELS.
- 11. PROVIDE ADEQUATE BOLSTER, HI-CHAIRS, SUPPORT BARS, ETC., TO MAINTAIN MINIMUM SPECIFIED CLEAR CONCRETE COVER FOR THE ENTIRE LENGTH OF ANY AND ALL BARS. PROVIDE CONTINUOUS #4 SPACER BARS IN WALLS AND SLABS TO SUPPORT DOWELS. HEAVIER SPACER BARS MAY BE REQUIRED IN BEAMS TO ALLEVIATE CONGESTION IN THE PRESENCE OF TENDON BUNDLES AND MILD STEEL REINFORCEMENT IN THE SAME OR IN PARALLEL LAYERS.
- 12. SPLICES OF REINFORCEMENT SHALL BE MADE ONLY WHERE REQUIRED OR PERMITTED ON THE CONTRACT DRAWINGS OR IN PROJECT SPECIFICATIONS, OR SPECIFICALLY AUTHORIZED BY THE ENGINEER. WHEREVER PERMITTED, SPLICING MAY BE OF MECHANICAL TYPE OR LAP SPLICE TYPE, EXCEPT LAP SPLICES IN DRILLED PIERS SHALL NOT BE PERMITTED. MECHANICAL SPLICES SHALL BE CAPABLE OF DEVELOPING 125% OF BAR YIELD STRENGTH. FOR MECHANICALLY SPLICED OR LAP SPLICED DISSIMILAR BARS USE SMALLER BAR YIELD STRENGTH OR TABULATED LAP LENGTH RESPECTIVELY TO DETERMINE MECHANICAL SPLICER STRENGTH OR LAP LENGTH. SPLICE PLACEMENT SHALL BE DONE AT POINTS OF MINIMUM STRESS AS MUCH AS POSSIBLE. WHERE NOT SPECIFICALLY INDICATED, DOWELS SHALL MATCH SIZE, NUMBER AND SPACING OF THE MAIN REINFORCEMENT ONTO WHICH THEY ARE SPLICED. UNLESS NOTED OTHERWISE.
- 13. ALL REINFORCEMENT BAR LAP SPLICES SHALL CONFORM TO THE LAP SPLICE SCHEDULES BELOW. ALL WELDED WIRE FABRIC SHALL BE LAPPED TWO (2) FULL MESH PANELS AND TIED SECURELY.

STANDARD HOOKED BAR DEVELOPMENT LENGTH Ldh SCHEDULE									
f _c ', PSI	4,0	00	5,0	000	7,000				
C - COATED U - UNCOATED	С	U	С	U	С	U			
SIZE									
#4	8"	7"	8"	6"	6"	6"			
#5	10"	9"	9"	8"	8"	7"			
#6	12"	10"	11"	9"	9"	8"			
#7	14"	12"	13"	11"	11"	9"			
#8	16"	14"	15"	12"	12"	10"			
#9	18"	15"	17"	14"	14"	12"			
#10	21"	17"	18"	15"	16"	13"			
#11	23"	19"	20"	17"	17"	15"			



ACI STD. 90° HOOKED BAR
DEVELOPMENT SCHEMATIC

SCALE: NONE

HOOKED BAR DEVELOPMENT TABLE NOTES:

1. SIDE COVER (NORMAL TO PLANE OF HOOK) SHALL NOT BE LESS
THAN 2 1/2". OTHERWISE INCREASE TABULATED VALUES BY A FACTOR
OF 1.4.

2. STANDARD HOOKED BAR (ACI 318-14 TAB 25.3.1) DEVELOPMENT LENGTHS IN SHADED COLUMNS (FOR COATED BARS) MAY BE USED FOR UNCOATED BARS OF THE SAME SIZE. AT CONTRACTOR'S OPTION.

CONCRETE NOTES (CONT.)

					SCH	IEDUL	E NO.	1 - UN	COA	TED TO	OP BA	RS					
BAR SIZE		#4 #5		5	#6		#7		#8		#9		#10		#11		
CLASS	S: LAP	Α	В	Α	В	Α	В	Α	В	Α	В	Α	В	Α	В	Α	В
f'c	4000	15	20	24	31	34	44	54	70	62	80	70	90	80	104	87	11
(psi)	5000	14	18	21	27	30	39	48	63	55	72	62	81	70	91	78	10
	6000	13	16	19	25	28	36	39	51	51	66	55	72	65	85	71	92
	8000	12	14	17	22	24	31	34	44	44	57	48	62	57	74	62	80

		_														
BAR	SIZE	#	4	#5	5	#6	6	#7	,	#8	3	#9)	#1	0	#1
CLAS	S: LAP	А	В	Α	В	Α	В	Α	В	Α	В	Α	В	Α	В	Α
f'c	4000	12	15	18	24	26	34	37	48	48	62	52	68	62	80	67
(psi)	5000	12	14	16	21	23	30	33	43	43	56	47	61	55	72	60
	6000	12	13	15	19	21	28	30	39	39	51	43	55	50	65	55
	8000	12	12	13	17	19	24	26	34	34	44	37	48	44	57	48

SCHEDULE NO. 3 - EPOXY COATED TOP BARS																	
BAR	SIZE	#	4	#5	5	#6	6	#7	,	#8	3	#9)	#1	0	#1	1
CLAS	S: LAP	Α	В	Α	В	Α	В	Α	В	Α	В	Α	В	Α	В	Α	В
f'c	4000	20	26	31	40	44	57	62	81	81	105	88	115	105	136	114	14
(psi)	5000	18	23	28	36	39	51	56	72	73	94	79	103	94	121	102	13
•	6000	16	21	25	33	36	47	51	66	66	86	72	94	85	111	93	12
	8000	14	18	22	28	31	41	44	57	58	75	63	81	74	96	81	10

SCHEDULE NO. 4 - EPOXY COATED BARS OTHER THAN TOP BARS																	
BAR	SIZE	#	4	#5	5	#6	6	#7	7	#8	3	#9)	#1	0	#1	1
		Α	В	Α	В	Α	В	Α	В	Α	В	Α	В	Α	В	Α	В
fc (psi)	4000	18	23	27	35	39	50	55	71	72	93	78	101	92	120	100	130
	5000	16	20	24	32	35	45	49	64	64	83	70	91	83	107	90	11
	6000	14	19	22	29	32	41	45	58	59	76	64	83	75	98	82	100
	8000	13	16	19	25	28	36	39	51	51	66	55	72	65	85	71	92

THE FOLLOWING NOTES SHALL APPLY TO THE ABOVE SCHEDULES:

- LINEAR INTERPOLATION BETWEEN VALUES ABOVE IS PERMITTED.
 REINFORCEMENT IN CONCRETE MEMBERS COVERED BY THESE NOTES SHALL BE INSTALLED TO A MINIMUM CLEAR CONCRETE COVER IN ACCORDANCE WITH TABLE
- UNDER ITEM (9) HEREIN ABOVE, UNLESS OTHERWISE INDICATED ON THE DRAWINGS.
 FOR REBAR SPLICES IN COLUMNS REFER TO SPECIAL COLUMN REBAR PLACEMENT
- DETAILS.
 LAP SPLICING OF BARS LARGER THAN #11 IS NOT PERMITTED.

NOTED OTHERWISE.

- LAP SPLICING OF BARS LARGER THAN #TT IS NOT PERMITTED.
 CLEAR SPACING BETWEEN BARS IN A LAYER OR CLEAR SPACING BETWEEN LAYERS
 OF BARS BEING DEVELOPED IN BEAMS, SHALL NOT BE LESS THAN THAT INDICATED
 UNDER ITEM (8) HEREIN ABOVE.
- CLEAR SPACING BETWEEN BARS BEING DEVELOPED IN WALLS AND SLABS, SHALL NOT BE LESS THAN TWO (2) TIMES BAR DIAMETER (SMALLER OF THE TWO IN A LAP SPLICE, IF DISSIMILAR).
- FOR PURPOSES OF DEFINING TOP BARS, THOSE INDICATED ON THE CONTRACT DRAWINGS AS "TOP" BARS SHALL BE DETAILED ACCORDINGLY. OTHERWISE, THOSE BARS WHERE MORE THAN TWELVE (12) INCHES OF FRESH CONCRETE IS CAST BELOW THE DEVELOPMENT LENGTH OR SPLICE SHALL BE CONSIDERED TOP BARS. EXCEPTION TO THIS SHALL BE WALL HORIZONTAL REINFORCEMENT WHERE ALL BARS SHALL BE CONSIDERED "OTHER THAN TOP BARS".
- CLASS "A" SPLICE SHALL BE USED ONLY WHERE SPECIFICALLY REFERENCED ON THE CONTRACT DRAWINGS.
- WHEN REFERENCED ON THE DRAWINGS, FOR TENSION DEVELOPMENT LENGTH USE CLASS "A" LENGTH FROM THE ABOVE SCHEDULES.
- 14. TENSION DEVELOPMENT LENGTH OF BARS WITH STANDARD HOOKS IS TABULATED ON THIS DRAWING. SEE TABLE AND DETAIL 1 BELOW.
- 15. ADDITIONAL REINFORCING BARS SHALL BE PROVIDED AROUND ALL SLAB-ON-GRADE, ELEVATED FLOOR AND WALL OPENINGS IN ACCORDANCE WITH TYPICAL DETAILS, UNLESS
- 16. THERE SHALL BE NO FIELD CUTTING OF ANY REINFORCEMENT WITHOUT AN EXPRESSED WRITTEN CONSENT OF THE ARCHITECT. FIELD MODIFICATIONS TO APPROVED SHOP DRAWINGS SHALL NOT BE DONE WITHOUT ARCHITECT'S WRITTEN APPROVAL. ALL FIELD BENDING OF REINFORCEMENT SHALL BE APPROVED IN WRITING BY THE ARCHITECT, UNLESS EXPRESSLY PERMITTED ON THE CONTRACT DRAWINGS. ALL FIELD BENDING OF REINFORCEMENT SHALL BE DONE COLD. HEATING OF BARS WILL NOT BE PERMITTED. FIELD BENDING OF REINFORCEMENT LARGER THAN #5 IS NOT PERMITTED, UNLESS SPECIFICALLY APPROVED IN WRITING BY THE ARCHITECT.
- 17. ALL WALLS AND STRUCTURAL SLABS SHALL BE REINFORCED. IN THE EVENT NO REINFORCEMENT HAS BEEN INDICATED ON THE DRAWINGS, PROVIDE A MINIMUM OF #4@ 12" EACH WAY, EACH FACE.
- 18. NO CONSTRUCTION JOINT SHALL BE MADE WITHOUT ADDITIONAL REINFORCEMENT INSTALLED PERPENDICULAR TO THE PLANE OF THE JOINT. REFER TO TYPICAL DETAILS ON THE CONTRACT DRAWINGS. THE FOLLOWING QUANTITIES ARE MINIMUM IN PERCENT OF CROSS-SECTIONAL AREA OF THE CONCRETE ELEMENT IN THE PLANE OF THE CONSTRUCTION JOINT, UNLESS SPECIFICALLY NOTED OTHERWISE IN SECTIONS AND/OR DETAILS

STRUCTURAL COMPONENT	LOCATIONS	PERCENTAGE	MINIMUM TRANSVERSE
SLABS	TOP & BOTTOM	0.20%	
BEAMS	TOP & BOTTOM	0.33%	
BEAMS	STIRRUPS, EACH SIDE OF VERTICAL JOINT		5-#4@8"
COLUMNS	VERTICAL	1.00%	
COLUMNS	HORIZONTAL TIES ABOVE AND BELOW HORIZONTAL JOINT		5-#4@4"
WALLS	VERTICAL AND HORIZONTAL		SEE TYPICAL DETAILS

CONCRETE NOTES (CONT.)

- 19. CONSTRUCTION AND CONTROL JOINTS IN STRUCTURES SHALL BE PROVIDED IN ACCORDANCE WITH ACI 301-16 ARTICLE 2.2.2.5 AND AS FOLLOWS:
- A. FOR CONVENTIONAL CONSTRUCTION OF SLABS AND BEAMS. PLACE VERTICAL CONSTRUCTION JOINTS (BETWEEN SUCCESSIVE PLACEMENTS)
- WITHIN MIDDLE THIRD OF RESPECTIVE SPANS.

 B. HORIZONTAL CONSTRUCTION JOINTS IN WALLS SHOULD BE AVOIDED. WHEN REQUIRED FOR CONSTRUCTABILITY, PLACE WITHIN MIDDLE THIRD OF VERTICAL SPAN FOR WALLS SUPPORTED AGAINST HORIZONTAL
- TRANSLATION AT BOTH THE TOP AND BOTTOM ONLY.

 C. VERTICAL CONSTRUCTION JOINTS IN WALLS SHALL BE PROVIDED AS PER NOTE 11 UNDER "GENERAL FOUNDATION NOTES" ON SHEET S001, OR WHENEVER CONTROL JOINTS ARE PLACED. LOCATE WALL CONTROL/CONSTRUCTION JOINTS NO FARTHER APART THAN CLEAR WALL HEIGHT TIMES THREE(3) OR 25 FEET, WHICHEVER IS LESS, BUT NOT LESS THAN 10 FEET ON-CENTER. FOLLOW TYPICAL DETAILS FOR JOINT GROOVES
- AT ALL CONSTRUCTION AND CONTROL JOINTS.

 D. PROVIDE CONTROL JOINTS CONSTRUCTED WITH THE USE OF CHAMFER STRIPS IN ALL UPTURNED BEAMS AND "CRASH" WALLS. EXTEND JOINT OVER THE TOP OF UPTURNED BEAM OR "CRASH" WALL, AND FILL WITH SEALANT. IF NOT SPECIFICALLY INDICATED, SPACE SUCH JOINTS AT 10 FT MAXIMUM ON-CENTER.
- E. SURFACE OF A MEMBER CROSS-SECTION AT A CONSTRUCTION JOINT SHALL BE INTENTIONALLY ROUGHENED TO A MINIMUM 1/4 INCH AMPLITUDE PRIOR TO PLACEMENT OF ADJACENT CONCRETE SEGMENT.
- F. FOR CONVENTIONAL REINFORCED CONCRETE CONSTRUCTION, STRUCTURE ON EITHER SIDE OF JOINT SHALL BE SHORED UNTIL THE AFFECTED MEMBER CONCRETE HAS ATTAINED MINIMUM SPECIFIED 28-DAY COMPRESSIVE STRENGTH.
- G. IN ALL INSTANCES, LOCATIONS OF ALL CONSTRUCTION JOINTS SHALL BE REVIEWED BY THE ARCHITECT. NO HORIZONTAL CONSTRUCTION JOINTS WILL BE PERMITTED IN BEAMS, SLABS, MATS, OR PADS UNLESS SPECIALLY SHOWN ON THE DRAWINGS.
- H. ALL CONSTRUCTION JOINTS SHALL BE WIRE BRUSHED, CLEANED, MOISTENED AND TREATED WITH A CONCRETE SLURRY OR NEAT CEMENT GROUT IMMEDIATELY PRIOR TO PLACING NEW CONCRETE.
- 20. ISOLATION JOINTS: ALL SLABS-ON-GRADE SHALL BE ISOLATED FROM ADJACENT WALLS AND BUILDING COLUMNS, PIERS OR PILASTERS, PER TYPICAL DETAILS, UNLESS NOTED OTHERWISE.
- 21. WHERE HOOKED BARS ARE SHOWN ON THE DRAWINGS, PROVIDE ACI STANDARD 90° HOOKS UNLESS SPECIFICALLY SHOWN OR NOTED OTHERWISE.
- 22. PLACE ALL SLABS-ON-GRADE AND SLABS-ON-FILL WITH AN APPROVED PATTERN AND SEQUENCE OF CONTROL AND CONSTRUCTION JOINTS TO MINIMIZE SHRINKAGE CRACKING. REFER TO TYPICAL DETAILS, NOTES AND SUGGESTED JOINT ARRANGEMENT SHOWN ON THESE DRAWINGS.
- 23. PROVIDE WATERSTOPS AT ALL WALL AND MAT CONTRACTION (CONTROL) AND CONSTRUCTION JOINTS BELOW EXTERIOR GRADE AS SHOWN ON THE DRAWINGS. REFER TO THE PROJECT SPECIFICATIONS FOR MORE INFORMATION.



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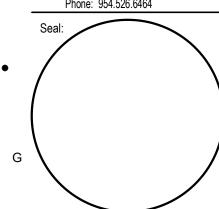
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STRUCTURAL PRECAST CONCRETE NOTES

- 1. ALL STRUCTURAL PRECAST CONCRETE SHALL BE DESIGNED FOR THE SPAN AND LOADING CONDITIONS SHOWN ON THE DRAWINGS INCLUDING BUT NOT LIMITED TO THOSE IMPOSED BY THE ERECTION SEQUENCES AND METHODS, BY A PRECAST MANUFACTURER'S PROFESSIONAL ENGINEER CURRENTLY LICENSED TO PRACTICE IN THE STATE WHERE THE PROJECT IS LOCATED. ALL DESIGN CALCULATIONS, INCLUDING THE ANALYSIS AND DESIGN FOR LATERAL AND GRAVITY LOADS AND THE DESIGN OF ALL STRUCTURAL ELEMENTS AND CONNECTIONS, SHALL BE SUBMITTED TO THE ARCHITECT FOR REVIEW PRIOR TO THE START OF FABRICATION.
- 2. DETAILED AND CHECKED SHOP DRAWINGS SHOWINGS ALL STRUCTURAL ELEMENTS, ERECTION DETAILS AND CONNECTIONS SHALL BE SUBMITTED TO THE ARCHITECT FOR REVIEW PRIOR TO THE START OF FABRICATION.
- 3. THE PRECAST CONCRETE MANUFACTURER SHALL BE RESPONSIBLE FOR FULL COORDINATION OF ALL ARCHITECTURAL, MECHANICAL, ELECTRICAL AND PLUMBING DETAILS AS THEY AFFECT THE STRUCTURAL SYSTEM.
- 4. THERE SHALL BE NO FIELD CUTTING OF PRECAST ELEMENTS WITHOUT THE PRIOR REVIEW OF THE ARCHITECT AND SUBSEQUENT WRITTEN CONSENT BY THE PRECAST CONCRETE MANUFACTURER.
- 5. ALL DETAILING, FABRICATION AND PLACING OF REINFORCING BARS SHALL CONFORM TO THE ACI 318 "BUILDING CODE REQUIREMENTS FOR STRUCTURAL CONCRETE", ACI 301 "SPECIFICATIONS FOR STRUCTURAL CONCRETE" AND THE ACI 315 "MANUAL OF STANDARD PRACTICE FOR DETAILING REINFORCED CONCRETE STRUCTURES" IN ADDITION TO THE INDUSTRY STANDARDS ISSUED BY THE PRECAST/PRESTRESSED CONCRETE INSTITUTE, AS REFERENCED IN THE PROJECT SPECIFICATIONS.
- 6. FOR STRUCTURAL ELEMENTS WITH ARCHITECTURAL FEATURES SEE ARCHITECTURAL DRAWINGS AND SPECIFICATIONS. COMPLY WITH SPECIAL ARCHITECTURAL CONCRETE REQUIREMENTS TO INCLUDE LOCATION, MIX DESIGN, FORMWORK, FINISH, ETC.
- 7. THE CONTRACTOR SHALL COORDINATE PRECAST, CAST-IN-PLACE AND STRUCTURAL STEEL WORK WITH THE ARCHITECTURAL AND STRUCTURAL DRAWINGS AND PROVIDE ALL NECESSARY INSERTS, REGLETS, ETC. AS MAY BE REQUIRED.
- 8. CONCRETE FOR STRUCTURAL PRECAST COMPONENTS SHALL ATTAIN A MINIMUM COMPRESSIVE STRENGTH OF 6,000 PSI AT TWENTY-EIGHT (28) DAYS, AND THE WATER- TO-CEMENTITIOUS MATERIALS RATIO SHALL NOT BE GREATER THAN 0.38.
- 9. ALL REBAR REINFORCEMENT SHALL CONFORM TO ASTM A615, GRADE 60, UNLESS NOTED OTHERWISE. WHERE WELDING OF REINFORCEMENT BARS IS REQUIRED FOR EITHER INTERCONNECTION OR FOR WELDING TO STEEL EMBEDS OR SHAPES, PROVIDE ASTM A706 GRADE 60 OR ASM A615 GRADE 40 MATERIAL.
- 10. WHERE SPECIFICALLY NOTED FOR USE OF STAINLESS STEEL REINFORCEMENT, PROVIDE BARS CONFORMING TO ASTM A955 GRADE 60.
- 11. LAP SPLICING OF REINFORCEMENT SHALL BE AVOIDED. LAP SPLICING IN TENSION TIE MEMBERS SHALL NOT BE PERMITTED. SHOULD LAP SPLICING BE REQUIRED FOR CONSTRUCTABILITY, PLACE AT POINTS OF MINIMUM TENSILE OR SHEAR STRESS AND STAGGER PER ACI 318 SECTION 25.5. MECHANICAL OR WELDED SPLICES OF REINFORCEMENT SHALL BE PERMITTED WHEN IN COMPLIANCE WITH ACI 318 SECTION 25.5.7.1 AND STAGGERED A MINIMUM OF 24 INCHES. LAP SPLICING OF WELDED WIRE FABRIC SHALL BE PERMITTED WHEN DONE OVER AT LEAST TWO FULL MESH PANELS AND TIED SECURELY.
- 12. WELDING OF REINFORCEMENT SHALL CONFORM TO AWS D1.4 "STRUCTURAL WELDING CODE REINFORCING STEEL", LATEST EDITION. ALL WELDMENTS SHALL RECEIVE TWO (2) SHOP COATS OF PRIMER OR SHALL BE HOT-DIP GALVANIZED. WHERE WELDING OF STAINLESS STEEL IS REQUIRED IT SHALL CONFORM TO AWS D1.6 "STRUCTURAL WELDING CODE STAINLESS STEEL", LATEST EDITION.
- 13. ALL WELDED WIRE FABRIC SHALL CONFORM TO ASTM A185.
- 14. ALL PRESTRESSING STEEL SHALL CONFORM TO ASTM A416, STRESS RELIEVED, LOW RELAXATION, 7-WIRE STRAND, GRADE 250 OR 270, AT MANUFACTURER'S OPTION.
- 15. ALL STEEL EMBEDS SHALL CONFORM TO ASTM A36, EXCEPT WHERE STAINLESS STEEL IS SPECIFICALLY REQUIRED AS NOTED ON THE DRAWINGS. THE LATTER TO BE PROVIDED AS REQUIRED IN THE PROJECT SPECIFICATIONS.
- 16. ALL CONNECTION PLATES, INSERTS, BOLTS, NUTS AND WASHERS, COIL RODS AND COIL INSERTS SHALL BE OF MANUFACTURER'S STANDARD MATERIAL, HOT-DIP GALVANIZED, EXCEPT WHERE STAINLESS STEEL IS SPECIFICALLY REQUIRED AS NOTED ON THE DRAWINGS.
- 17. ALL DAMAGED GALVANIZING AS WELL AS FIELD WELDED CONNECTIONS SHALL BE CLEANED OF LOOSE AND DELETERIOUS MATERIALS AND TOUCHED UP WITH TWO (2) COATS OF ZINC-RICH PAINT IN ACCORDANCE WITH SSPC-PAINT 20.
- 18. DIAPHRAGM CONNECTIONS TO SPANDRELS VIA EMBEDS AND/OR WELD PLATES, THREADED COIL RODS OR CONICALLY THREADED DOWELS INTO CAST-IN MECHANICAL COUPLERS OR EQUIVALENT, ARE REQUIRED. MECHANICAL OR WELDED CONNECTIONS OF REBAR SHALL BE DESIGNED BY THE PRECAST CONCRETE MANUFACTURER AND SHALL CONFORM TO ACI 318 ARTICLE 25.5.7.1.
- 19. ALL GROUT SHALL BE OF THE NON-SHRINK, NON-FERROUS TYPE WITH MINIMUM f'c = 6,000 PSI 28-DAY COMPRESSIVE STRENGTH.

STRUCTURAL PRECAST CONCRETE NOTES (CONTINUED)

- 20. ERECTION SEQUENCE AND STRUCTURAL INTEGRITY AND STABILITY FOR ALL APPLICABLE LOADS SHALL BE COMPLETE AND SOLE RESPONSIBILITY OF THE PRECAST CONCRETE MANUFACTURER AND THEIR ERECTOR. THIS INCLUDES ALL MANNER OF BRACING REMOVAL SHOULD IT BE REQUIRED BY THE MANUFACTURER/ERECTOR. THE PRECAST CONCRETE MANUFACTURER SHALL NOTIFY THE ARCHITECT PROMPTLY BEFORE SUBMITTING THE SHOP DRAWINGS OF ANY LOADS IMPOSED UPON FOUNDATION DURING ERECTION IF THOSE LOADS EXCEED MAGNITUDES USED FOR THE STRUCTURE IN SERVICE. ARCHITECT RESERVES THE RIGHT TO REVIEW AND RE-DESIGN RESPECTIVE FOUNDATIONS OR ELEMENTS AND SUCH RE-DESIGN SHALL BE PAID FRO BY THE PRECAST CONCRETE MANUFACTURER AND/OR ERECTOR.
- 21. ALL ERECTION HARDWARE SHALL BE GALVANIZED UNLESS SPECIFICALLY APPROVED OTHERWISE BY THE ARCHITECT.
- 22. THE FOUNDATION FOR THE PRECAST STRUCTURE HAS BEEN DESIGNED TO SUPPORT THE COMPLETED SUPERSTRUCTURE. DURING ERECTION OF THE SUPERSTRUCTURE, THE CONTRACTOR SHALL FURNISH ALL TEMPORARY BRACING AND SUPPORTS AS THE RESULT OF THE CONTRACTOR'S CONSTRUCTION METHODS AND/OR SEQUENCE SO THAT STRUCTURAL INTEGRITY OF THE FOUNDATION IS NOT COMPROMISED.
- 23. SEE SPECIFICATIONS FOR ADDITIONAL REQUIREMENTS.

STRUCTURAL MASONRY NOTES

- 1. ALL MATERIAL AND WORKMANSHIP SHALL BE IN ACCORDANCE WITH THE SPECIFICATIONS FOR MASONRY STRUCTURES TMS 602-13.
- 2. MATERIALS:
- A. LOAD-BEARING CONCRETE MASONRY UNITS SHALL CONFORM TO ASTM C90, TYPE 1, WITH A MINIMUM UNIT COMPRESSIVE STRENGTH OF 2,650 PSI ON THE NET SECTION.
- B. LOAD-BEARING BRICK SHALL CONFORM TO ASTM C62 AND/OR ASTM C216 WITH A MINIMUM UNIT COMPRESSIVE STRENGTH OF 3,000 PSI.
- C. CONCRETE BUILDING BRICK SHALL CONFORM TO ASTM C55 WITH MINIMUM UNIT COMPRESSIVE STRENGTH OF 2,500 PSI AS AN AVERAGE OF THREE (3) BRICKS.
- D. MORTAR AND GROUT:
- a. MORTAR FOR REINFORCED AND NON-REINFORCED LOAD BEARING MASONRY SHALL CONFORM TO ASTM C270, PORTLAND - LIME, TYPE N BY PROPORTION, TO ACHIEVE A MINIMUM COMPRESSIVE STRENGTH OF 1,000 PSI.
- b. GROUT SHALL CONFORM TO ASTM C476, TO ACHIEVE A MINIMUM COMPRESSIVE STRENGTH OF 2,200 PSI.

E. MASONRY:

- a. COMPRESSIVE STRENGTH I'm OF ANY MASONRY SHALL BE DETERMINED BASED ON THE STRENGTH OF THE UNITS AND SHALL NOT BE LESS THAN 2.250 PSI.
- F. REINFORCING BARS:
- a. REINFORCING BARS FOR REINFORCED MASONRY SHALL CONFORM TO ASTM A615, GRADE 60, UNLESS NOTED OTHERWISE.
- 3. VERTICAL CELLS TO BE FILLED WITH GROUT SHALL BE ALIGNED TO PROVIDE A CONTINUOUS, UNOBSTRUCTED OPENING OF THE DIMENSIONS SHOWN ON THE PLANS. CELLS WHICH WILL CONTAIN VERTICAL REINFORCEMENT SHALL HAVE A MINIMUM OF TWO (2) INCH CLEAR OPENING.
- 4. GROUT FOR FILLING REINFORCED OR NON-REINFORCED CELLS SHALL BE FLUID WITH A 9" TO 11" SLUMP, AND PLACED BY ACCEPTABLE PRESSURE GROUTING PROCEDURES.
- 5. GROUT FOR FILLING REINFORCED OR NON-REINFORCED CELLS SHALL BE PLACED IN MAXIMUM FOUR (4) FOOT LIFTS AND CONSOLIDATED IN PLACE BY VIBRATION OR OTHER METHODS WHICH INSURE COMPLETE FILLING OF THE CELLS. ALL CELLS CONTAINING REINFORCING BARS AND/OR ANCHOR BOLTS SHALL BE FULLY GROUTED.
- 6. HOLLOW UNITS SHALL BE LAID WITH FULL MORTAR COVERAGE ON HORIZONTAL AND VERTICAL FACE SHELLS EXCEPT THAT WEBS SHALL ALSO BE BEDDED WHERE THEY ARE ADJACENT TO CELLS TO BE REINFORCED AND/OR FILLED WITH GROUT, IN THE STARTING COURSE ON FOOTINGS AND SOLID FOUNDATION WALLS AND IN NON-REINFORCED OR GROUTED PIERS, PILASTERS AND COLUMNS.
- 7. SOLID MASONRY UNITS SHALL BE LAID WITH FULL HEAD AND BED JOINTS.
- 8. POINTS OF BEARING SHALL BE ON TWO (2) COURSES OF SOLID MASONRY OR TWO (2) COURSES OF HOLLOW MASONRY GROUTED SOLID.
- 9. ALL CUTTING AND FITTING OF MASONRY, INCLUDING THAT REQUIRED TO ACCOMMODATE THE WORK OF OTHER TRADES, SHALL BE DONE WITH MASONRY SAWS.
- 10. CHASES SHALL BE BUILT INTO WALLS, NOT CUT IN. CHASES SHALL BE PLUMB AND SHALL BE A MINIMUM OF ONE (1) MASONRY UNIT LENGTH FROM JAMBS OF WALL OPENINGS. NO CHASES OTHER THAN THOSE SHOWN ON THE DRAWINGS SHALL BE CONSTRUCTED WITHOUT PRIOR REVIEW OF THE ARCHITECT.

STRUCTURAL MASONRY NOTES (CONTINUED)

- 11. REINFORCED MASONRY:
 - A. ALL WALLS AND PIERS SHALL HAVE HORIZONTAL JOINT REINFORCEMENTS AT 16" ON CENTER CONSISTING OF TWO (2) 9 GAGE RODS WITH 9 GAGE CROSS TIES AT 16" ON CENTER, GALVANIZED WITH 0.8 OZ. ZINC COATING, ASTM A116, CLASS 3. FOR COMPOSITE OR CAVITY WALLS PROVIDE TWO (2) RODS IN C.M.U. AND ONE (1) ROD IN FACE BRICK.
 - B. THE MINIMUM CLEAR DISTANCE BETWEEN PARALLEL BARS EXCEPT IN COLUMNS SHALL BE EQUAL TO THE NOMINAL DIAMETER OF THE BAR, BUT SHALL NOT BE LESS THAN 1".
 - C. VERTICAL REINFORCEMENT SHALL BE LAP SPLICED A MINIMUM OF 48 BAR DIAMETERS (NOT LESS THAN 1'-0") WHERE REQUIRED.
 - D. ALL BARS SHALL BE COMPLETELY EMBEDDED IN MORTAR OR GROUT. ALL BARS SHALL HAVE A COVERAGE OF MASONRY NOT LESS THAN:
 - 2" FOR BARS LARGER THAN #5
 1-1/2" FOR #5 BARS AND SMALLER.
 - 1. PROVIDE ADEQUATE, TEMPORARY BRACING AS REQUIRED DURING CONSTRUCTION TO WITHSTAND LATERAL LOADS AND THE PRESSURES OF FLUID GROUT
 - 2. CONCRETE MASONRY SHALL BE PROTECTED FROM ABSORBING MOISTURE AND WATER WHILE AT THE PLANT, DURING SHIPMENT AND AT THE SITE DURING CONSTRUCTION.
 - 3. ANCHORS, WALL PLUGS, ACCESSORIES AND OTHER ITEMS TO BE BUILT IN SHALL BE INSTALLED AS THE MASONRY WORK PROGRESSES. SEE ARCHITECTURAL DRAWINGS FOR ADDITIONAL DETAILS.
 - 4. FOR LINTEL NOTES AND SCHEDULE SEE DRAWING S003.
 - 5. SEE SPECIFICATIONS FOR ADDITIONAL REQUIREMENTS.



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Project:

1st Avenue S. Public Parking Garage

Owner:
Owner

H

Design Management

One Financial Plaza

100 SE 3rd Avenue, 10th Floor

Fort Lauderdale, Florida 33394

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Seal:

Project Phase:
Schematic Desi

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Project #: 11-22107.00-1

Sheet Issued: 09/22/22

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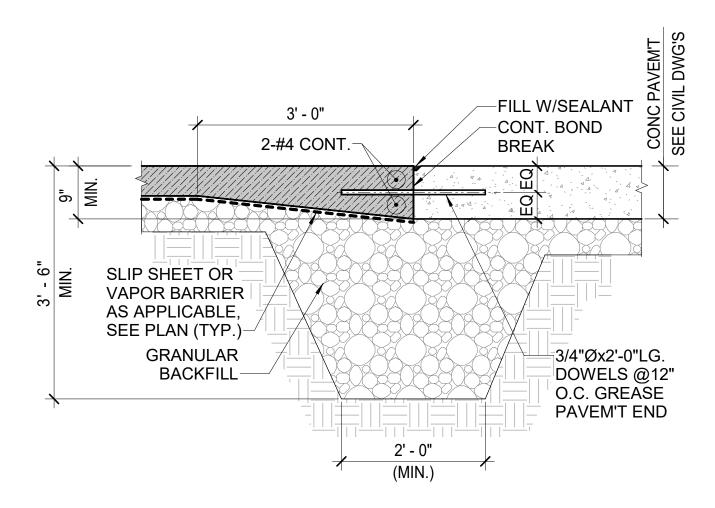
General Notes

Sheet No.

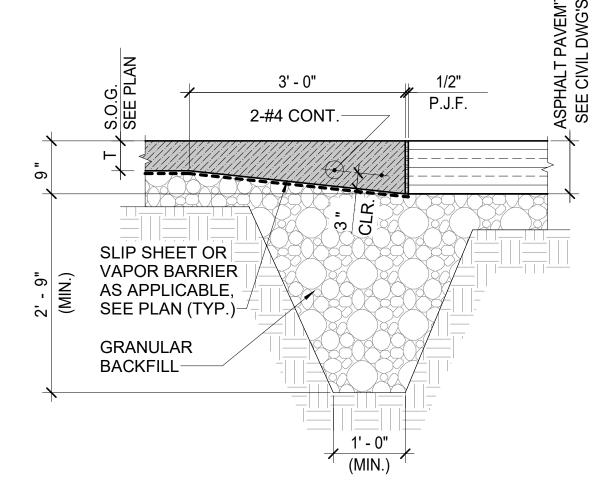
CENTERLINES ARE CONSIDERED OPTIMUM; HOWEVER, IT IS INTENDED THAT THE CONTRACTOR LAYOUT A PATTERN THAT WILL RESULT IN JOINTS AT 12'-0" MAXIMUM CENTER TO CENTER IN EACH DIRECTION INTENDED TO MINIMIZE CRACKING DUE TO CONCRETE DRYING SHRINKAGE AND THERMAL EFFECTS. DO NOT LOCATE CONSTRUCTION AND/OR CONTROL JOINTS BELOW MASONRY WALLS.

TYPICAL CONSTRUCTION JOINT

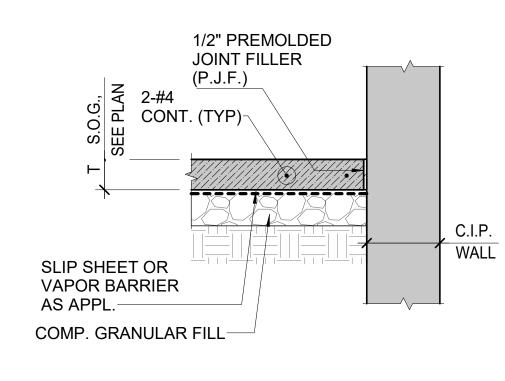
SLAB-ON-GRADE JOINT DETAILS FOR S.O.G. REINFORCED W/STRUCTURAL FIBERS ONLY S005 SCALE: N.T.S.



SLAB-ON-GRADE TO CONCRETE 4 PAVEM'T INTERFACE DETAIL S005 SCALE: N.T.S.

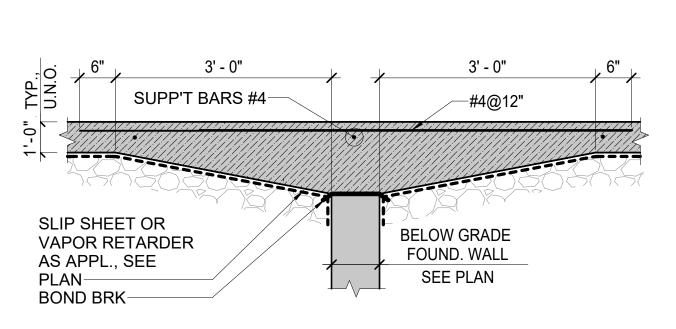


SLAB-ON-GRADE TO ASPHALT PAVEM'T INTERFACE DETAIL S005 SCALE: N.T.S.

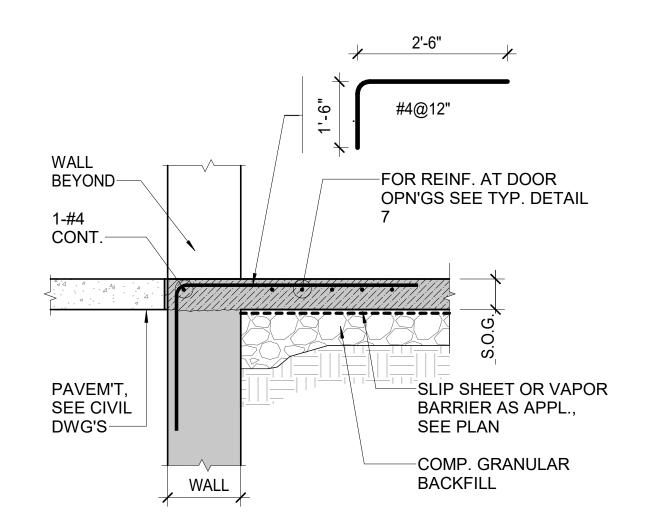


NOTE: PROVIDE CONTINUOUS SEALANT ALONG ALL SLAB TO WALL JOINTS.

TYP. SECTION SLAB-ON-GRADE AT FOUNDATION OR RETAINING WALL S005 SCALE: N.T.S.

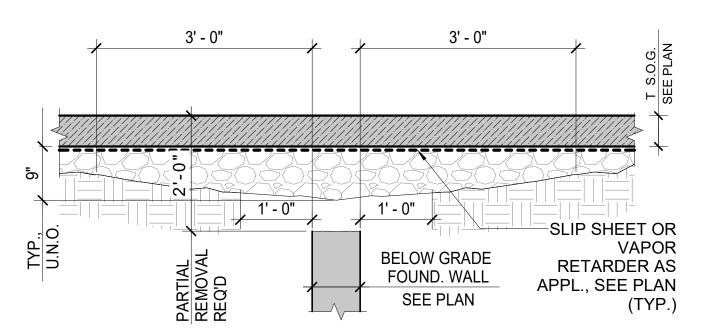


TYP. DETAIL OF SLAB-ON-GRADE AND 6 BURRIED WALL INTERFACE_OPTION 2 S005 SCALE: N.T.S.



NOTE:
PROVIDE CONTINUOUS SEALANT ALONG ALL
SLAB TO WALL JOINTS.

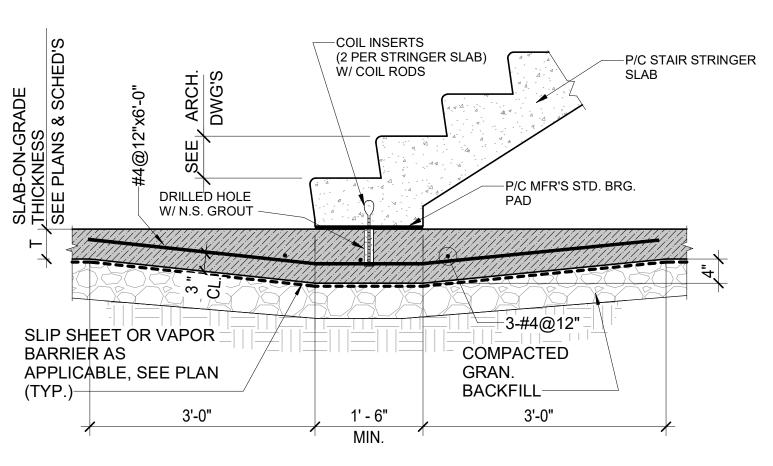
TYP. SECTION -SLAB-ON-GRADE AT FOUNDATION 3 OR RETAINING WALL AT DOOR OPENING S005 SCALE: N.T.S.



TYP. DETAIL OF SLAB-ON-GRADE AND BURRIED WALL INTERFACE _ OPTION 1 S005 SCALE: N.T.S.

EXT. PAVEM'T, SEE CIVIL 3 S005 DWG'S DOOR OPN'G U.N.O. **RE-ENTRANT CORNER** REBAR SEE TYP. DET.

TYP. SLAB-ON-GRADE-FOUNDATION-WALL 8 INTERFACE DETAIL AT DOOR OPENING S005 SCALE: N.T.S.



DETAIL AT BASE OF P/C STAIR S005 SCALE: N.T.S.

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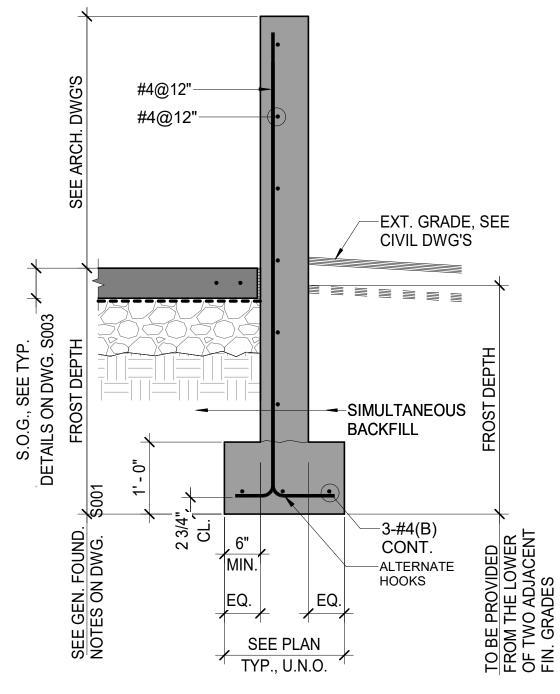
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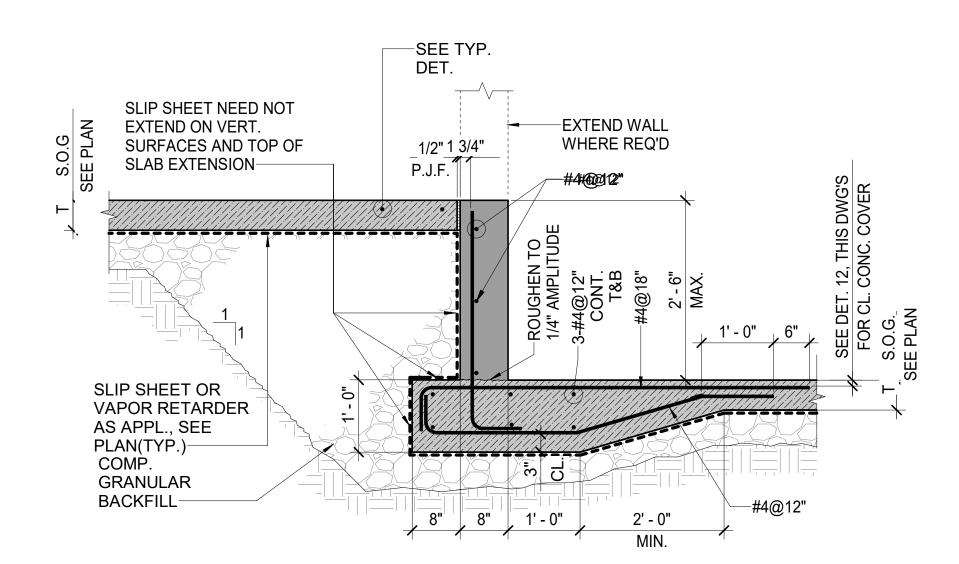
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Details

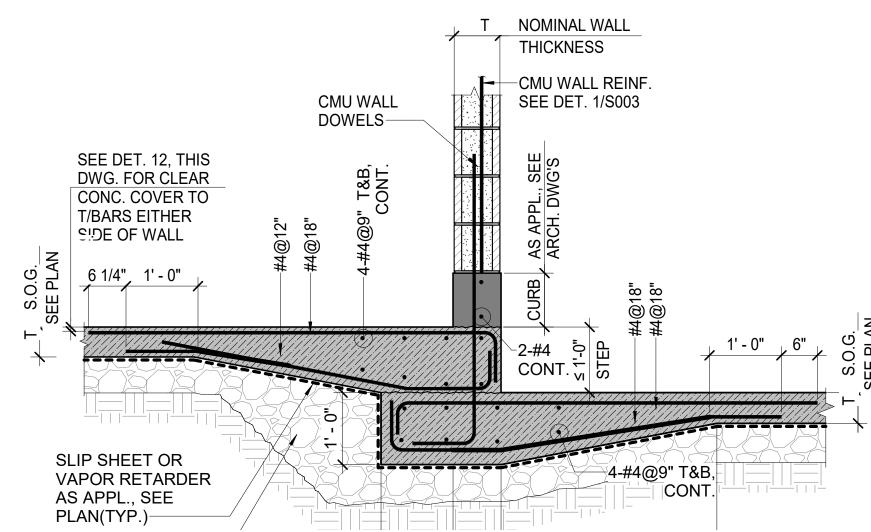
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TYP. EXTERIOR FOUND. WALL CROSS SECTION S006 SCALE: 3/4" = 1'-0"

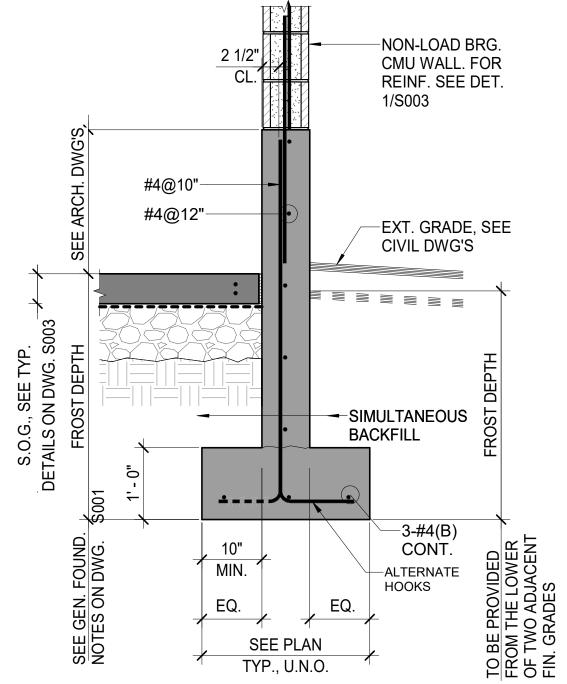


TYP. SLAB-ON-GRADE STEP DETAIL FOR DEPTH ≤ 2'-6" 4 AWAY FROM MASONRY WALLS S006 SCALE: N.T.S.

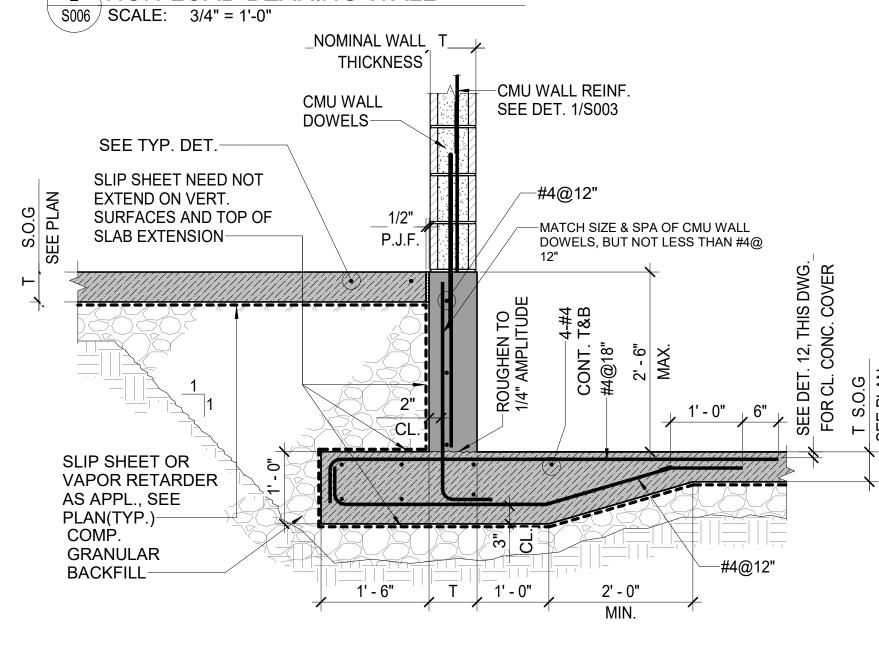


TYP. SLAB-ON-GRADE DEPRESSION DETAIL FOR DEPTH ≤ 7 1'-È0" ADJACENT TO MASONRY WALL

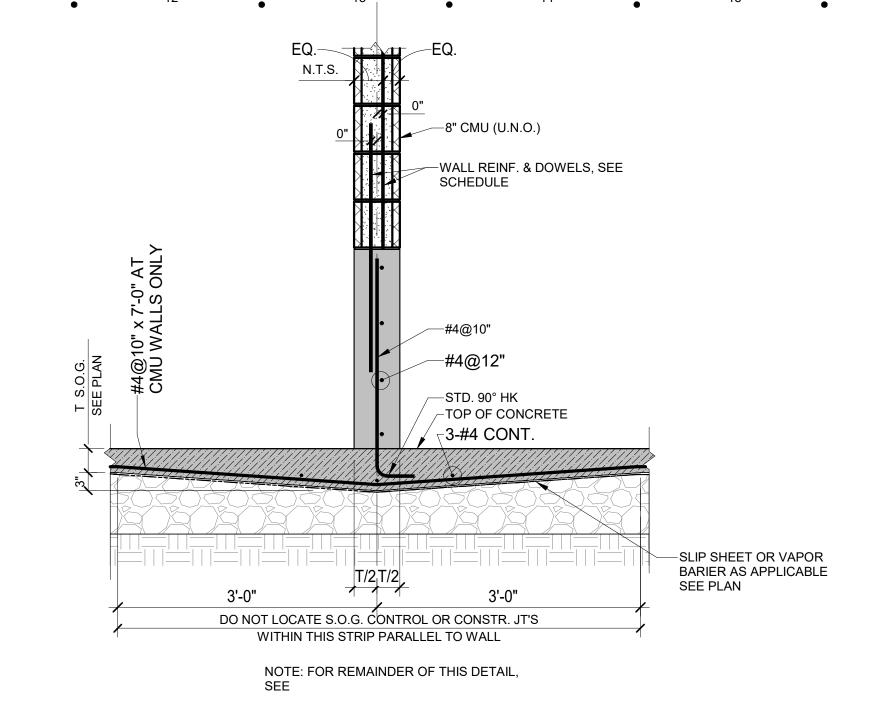
S006 SCALE: N.T.S.



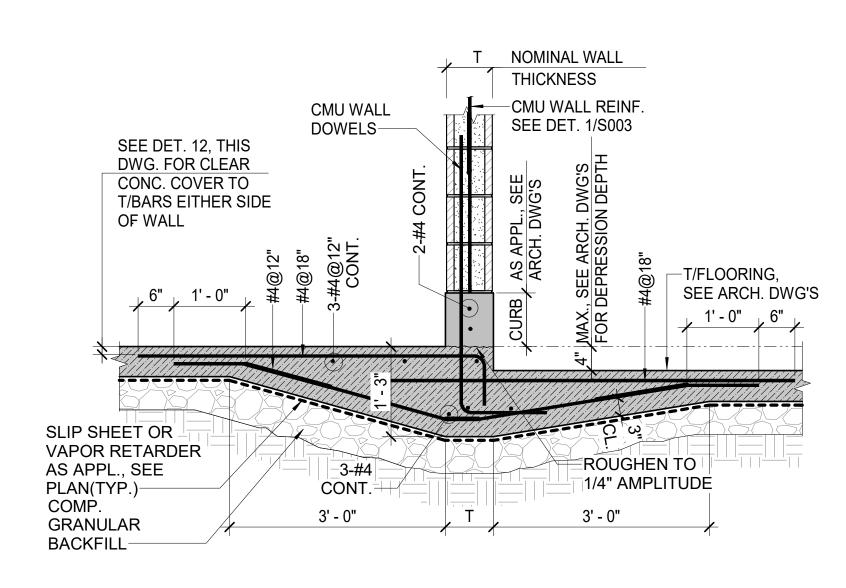
TYP. FOUND.WALL WITH CMU 2 NON-LOAD BEARING WALL



TYP. SLAB-ON-GRADE STEP DETAIL FOR DEPTH ≤ 2'-6" ADJACENT TO MASONRY WALLS S006 SCALE: N.T.S.



INTERIOR FOUND. WALL WITH CMU NON-LOAD BEARING WALL S006 SCALE: N.T.S.



TYP. SLAB-ON-GRADE DEPRESSION DETAIL FOR DEPTH ≤ 6 4" ADJACENT TO NON-LOAD BEARING MASONRY WALL S006 SCALE: N.T.S.



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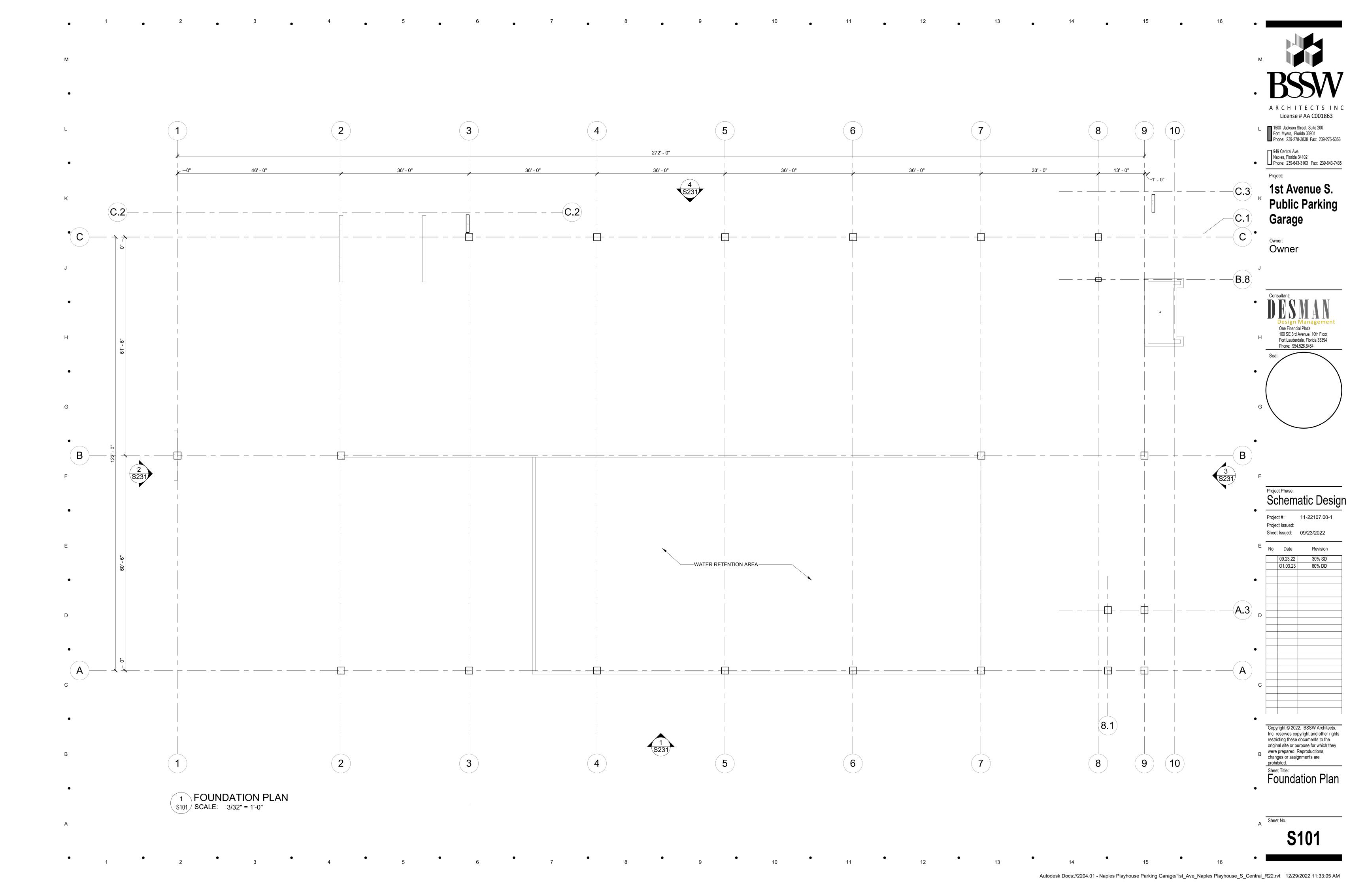
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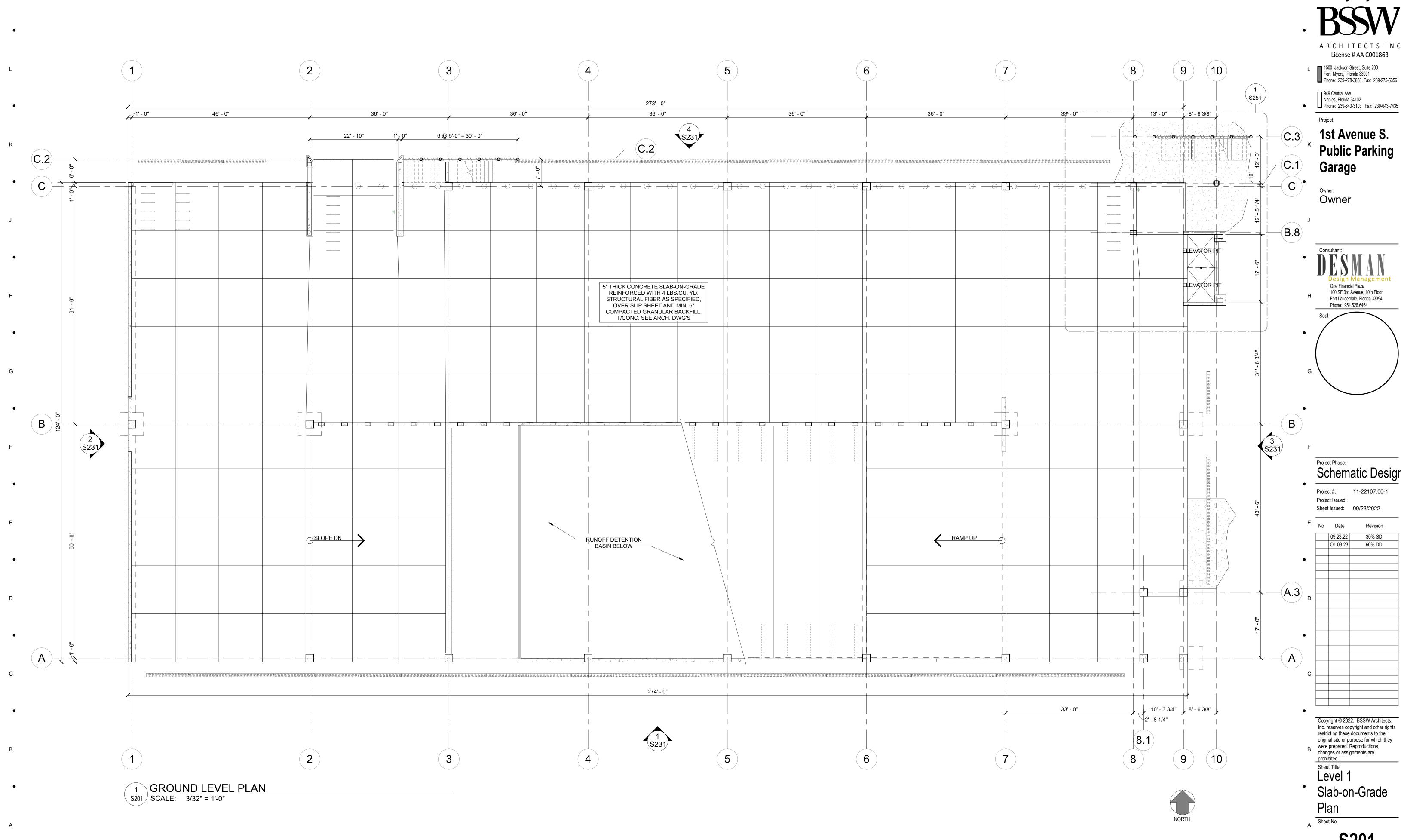
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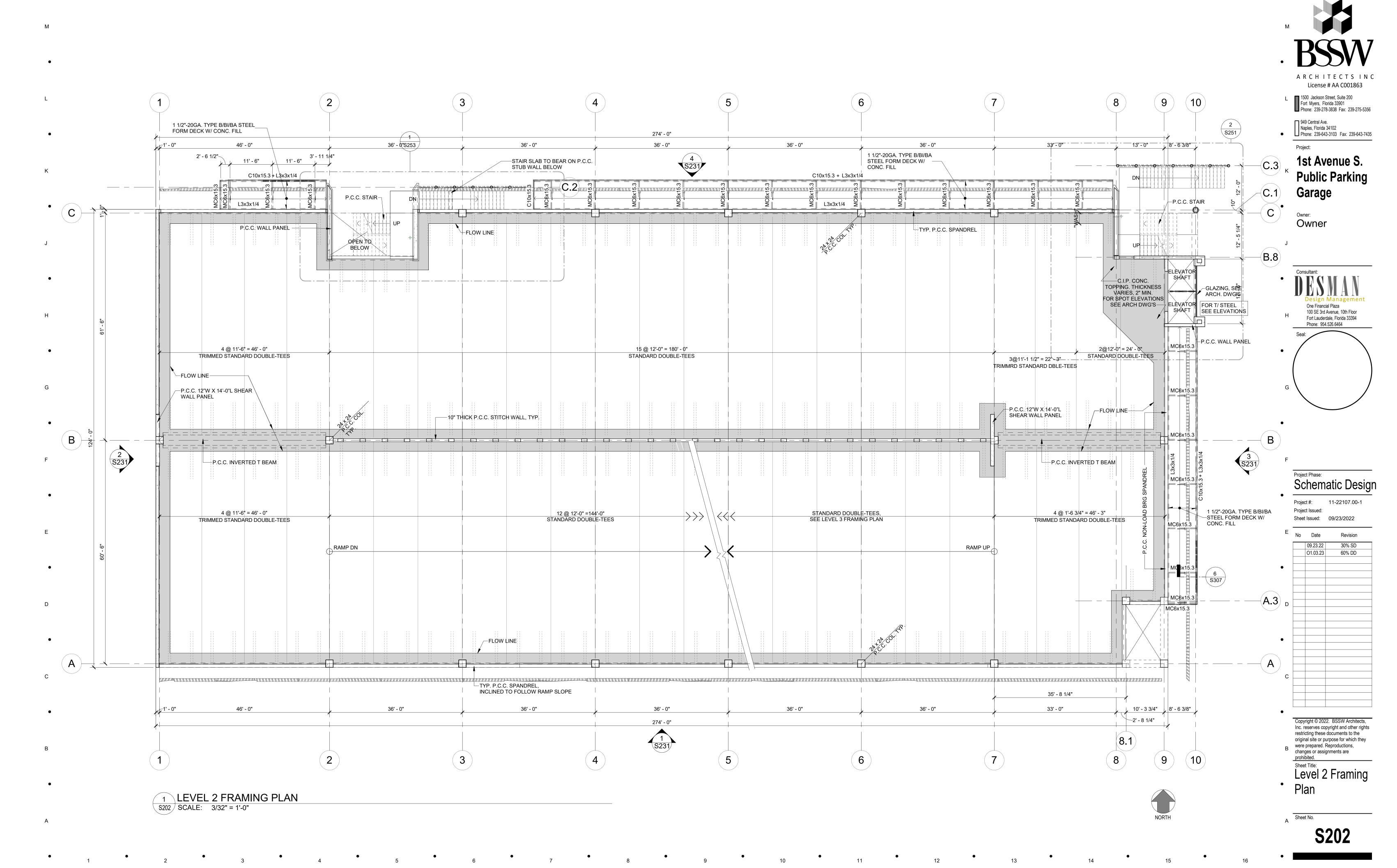
S006

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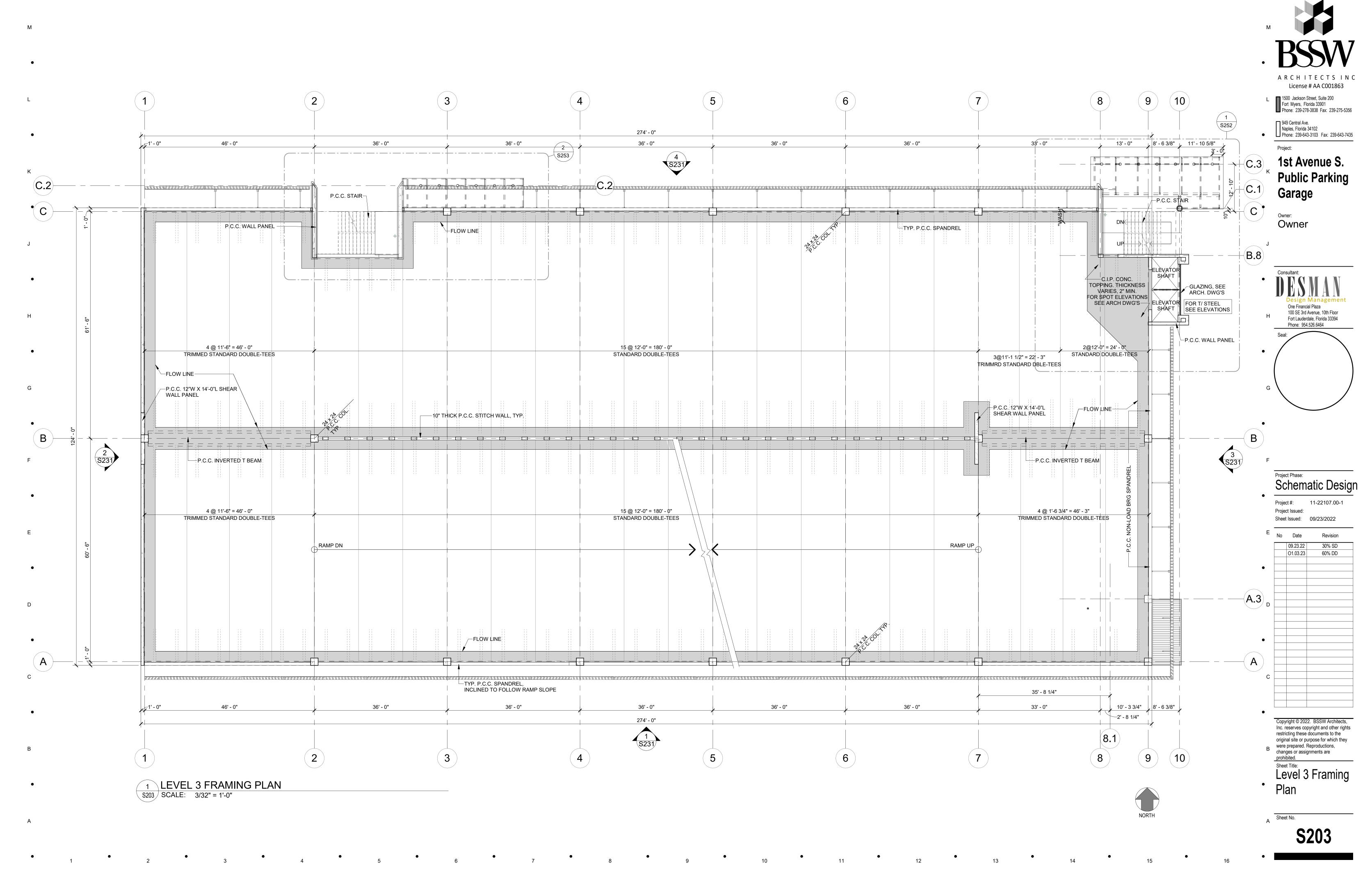




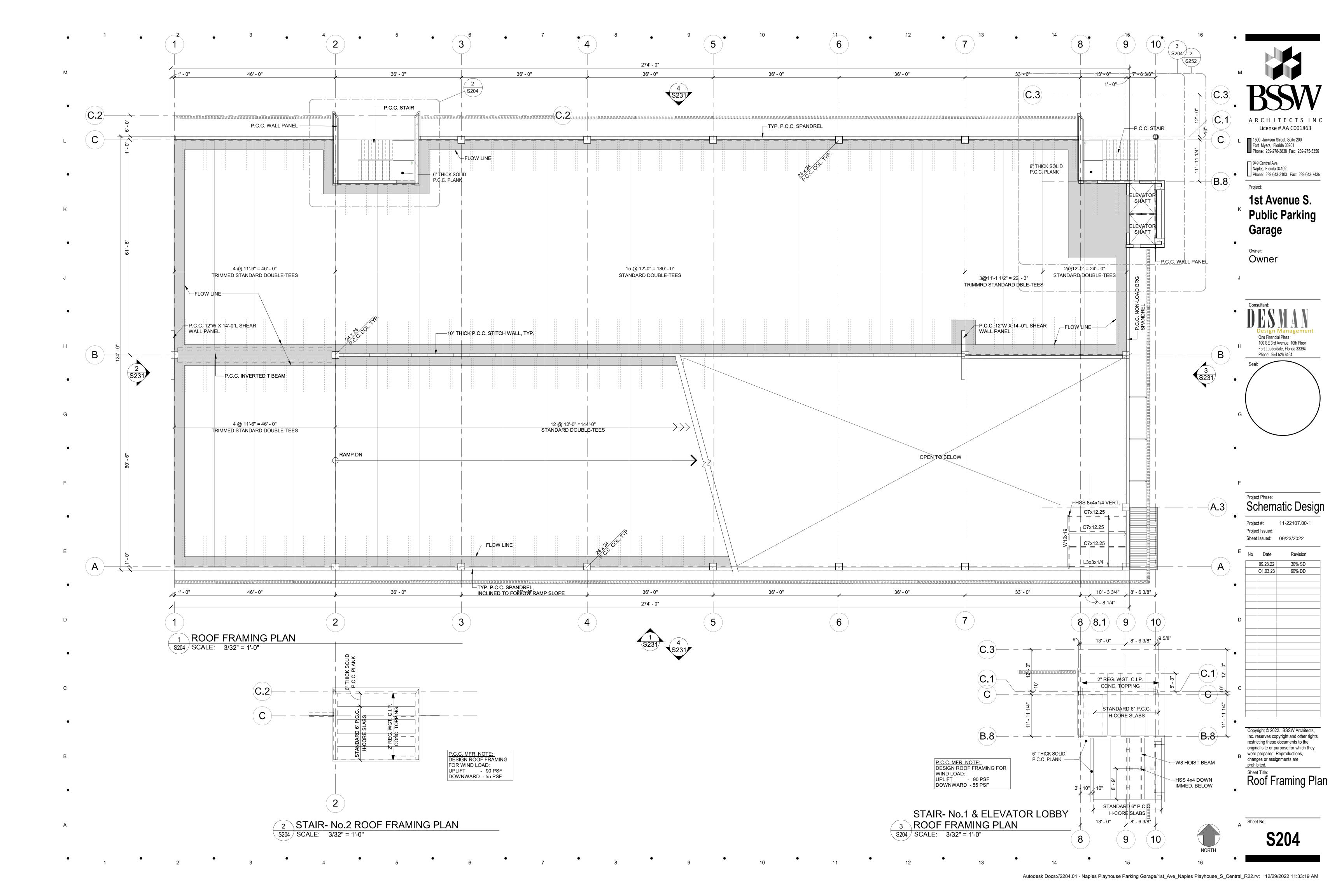
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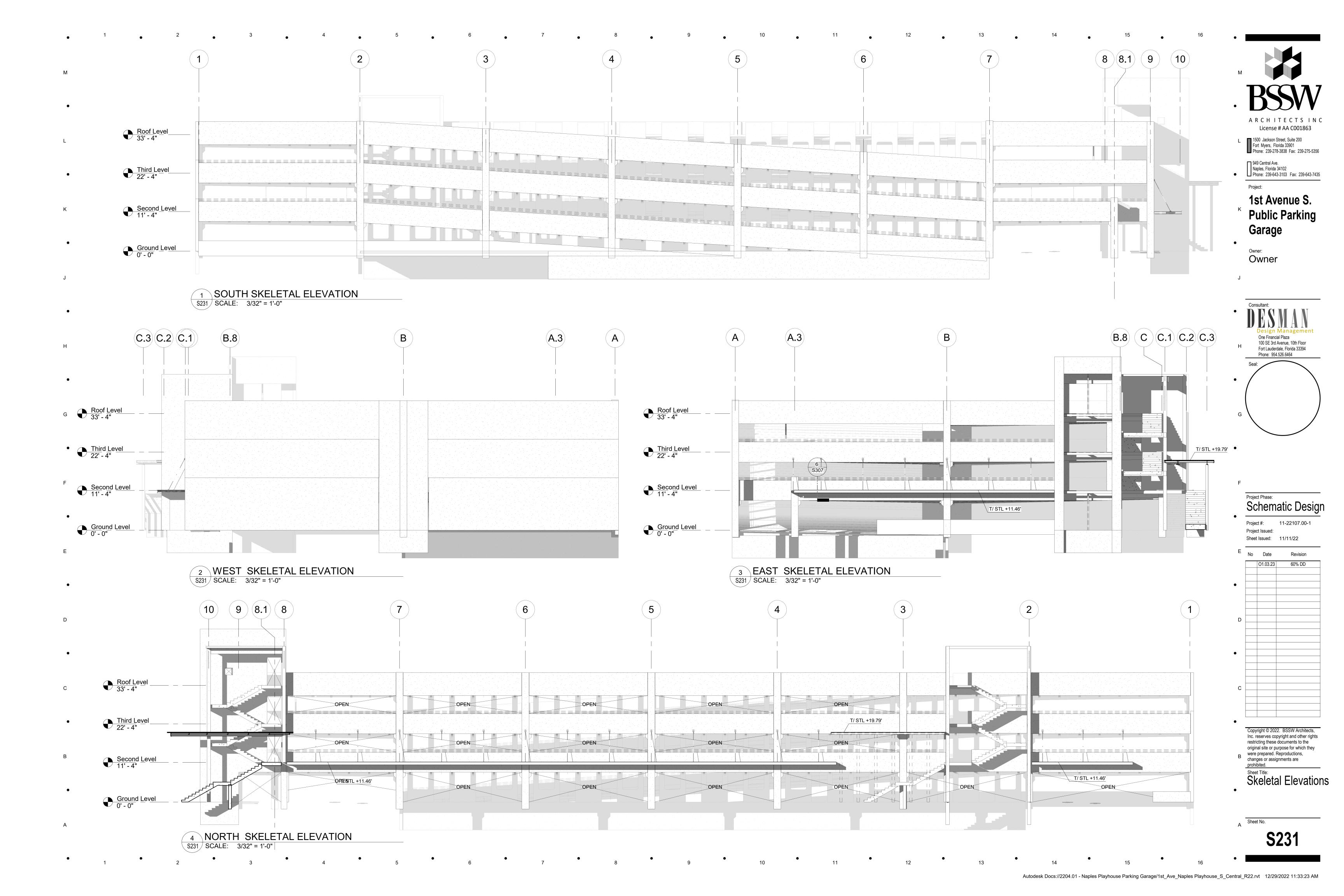


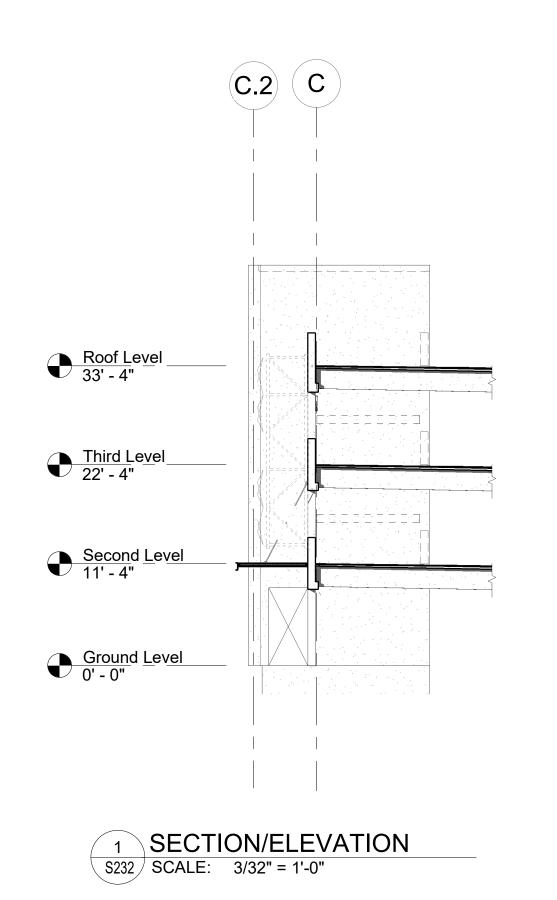
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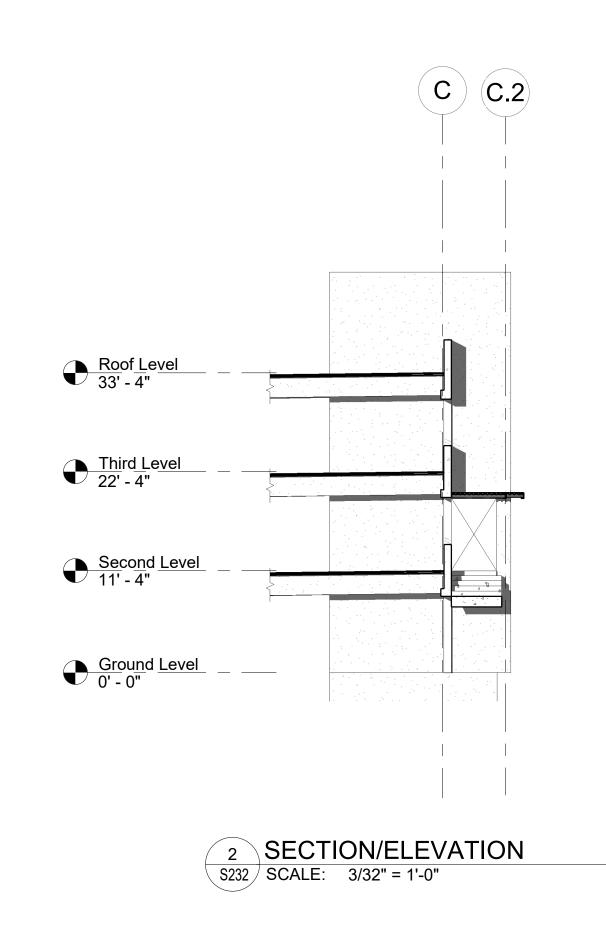


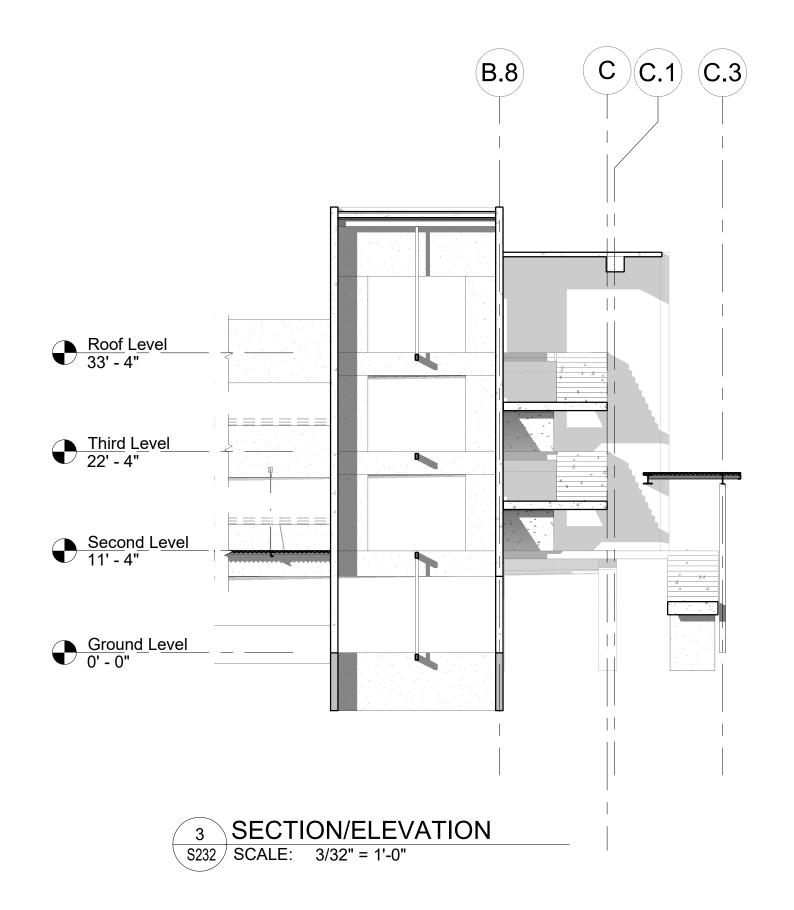
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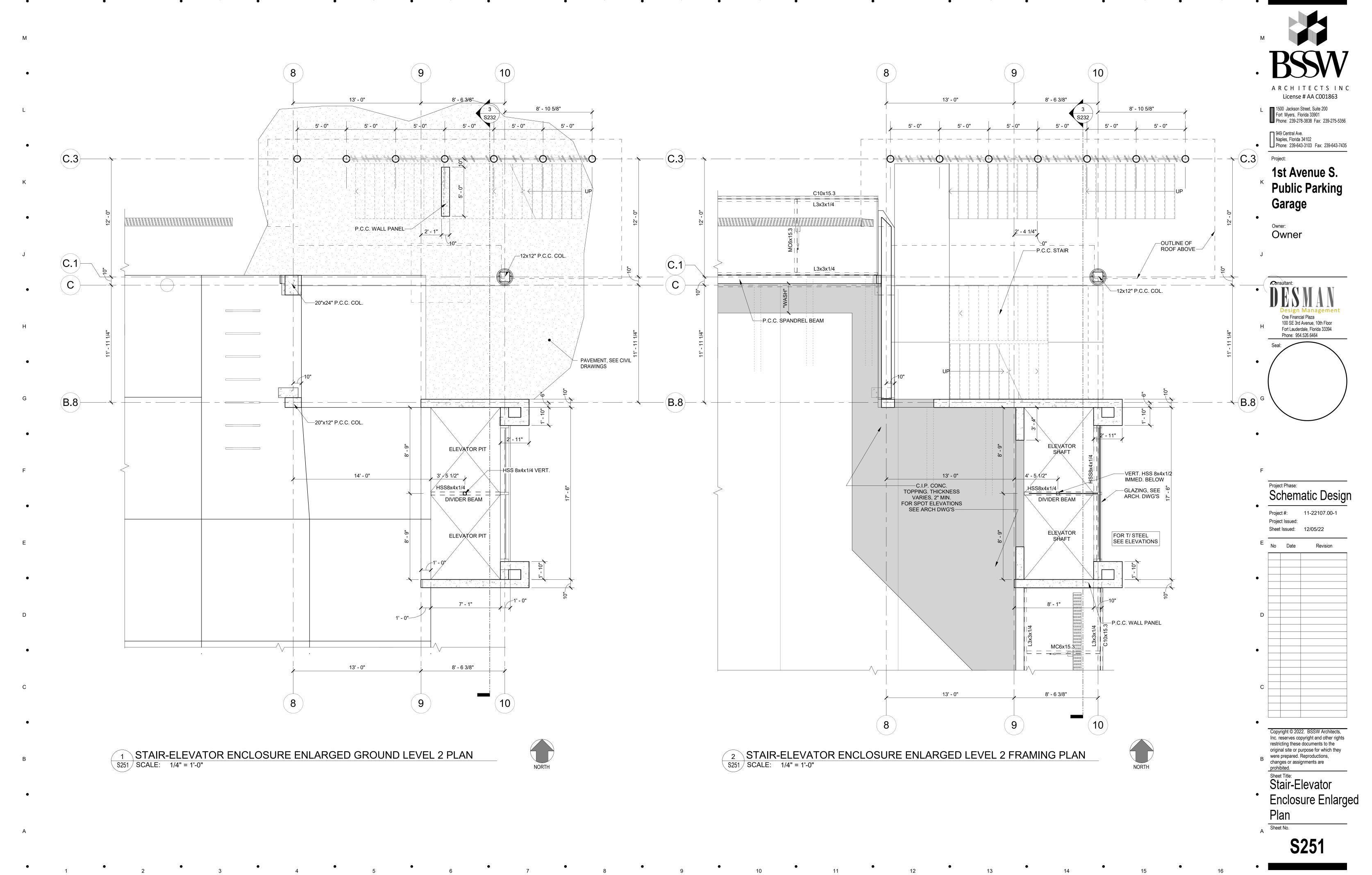
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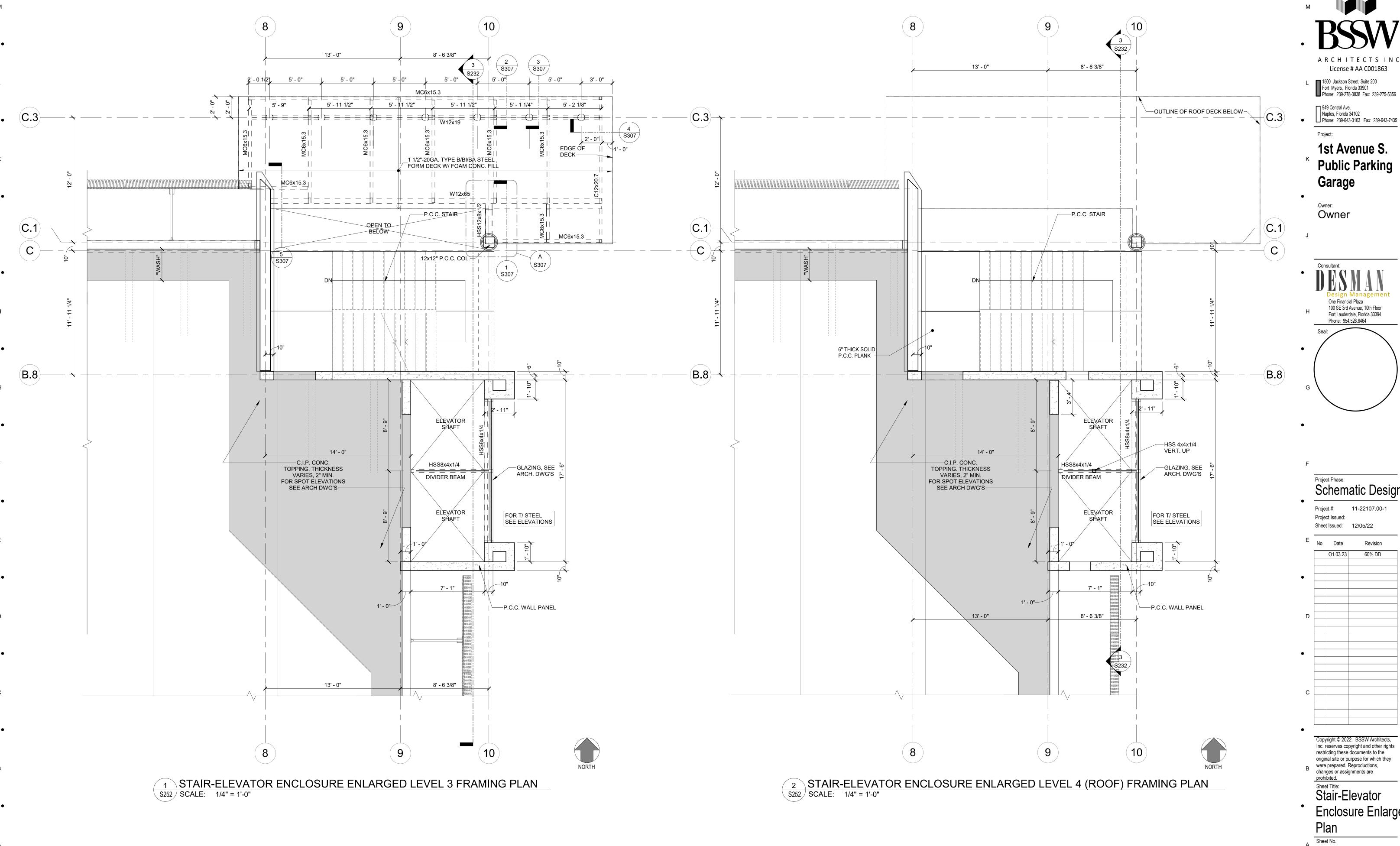
Skeletal Elevations and Sections

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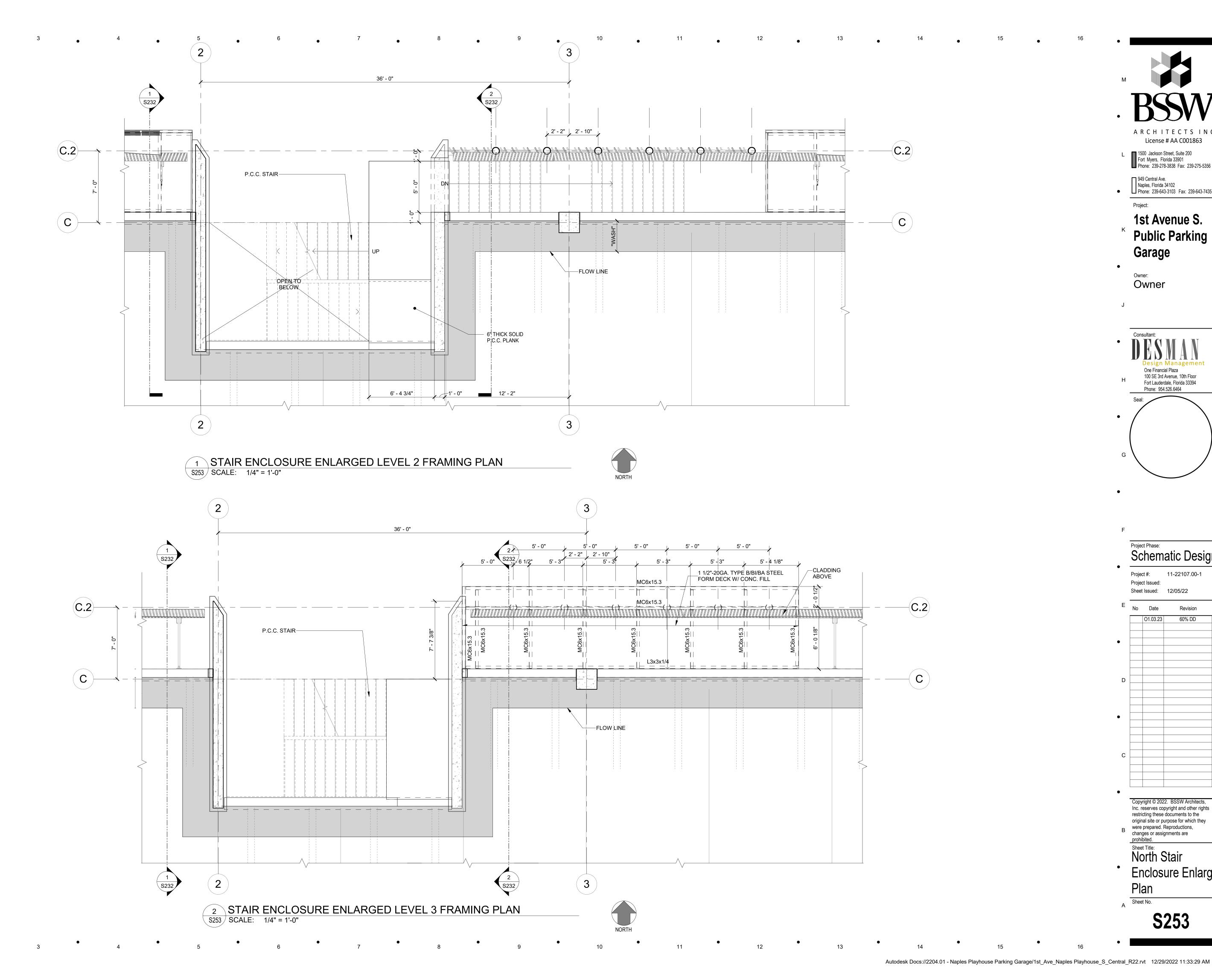
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Schematic Design

Enclosure Enlarged

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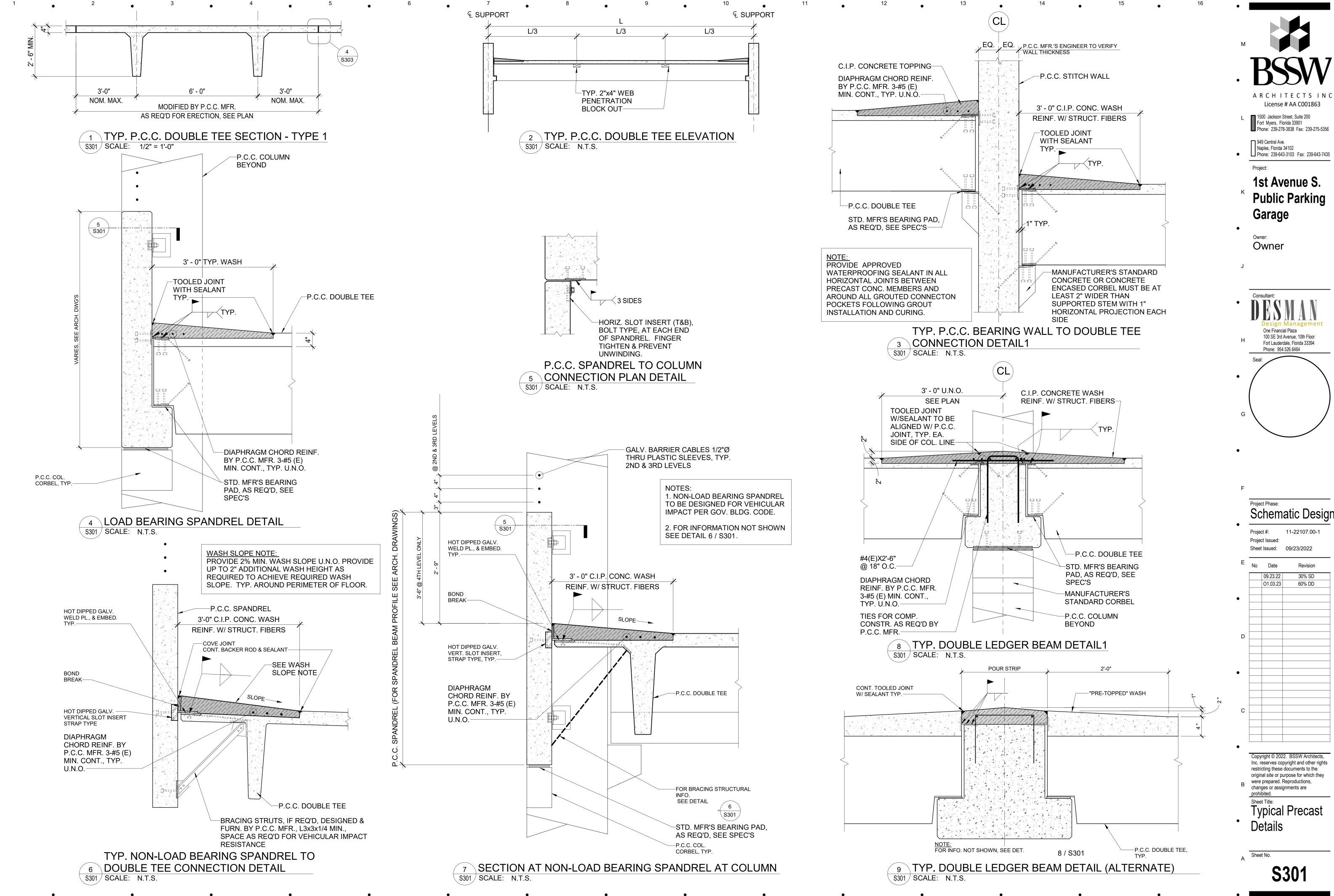
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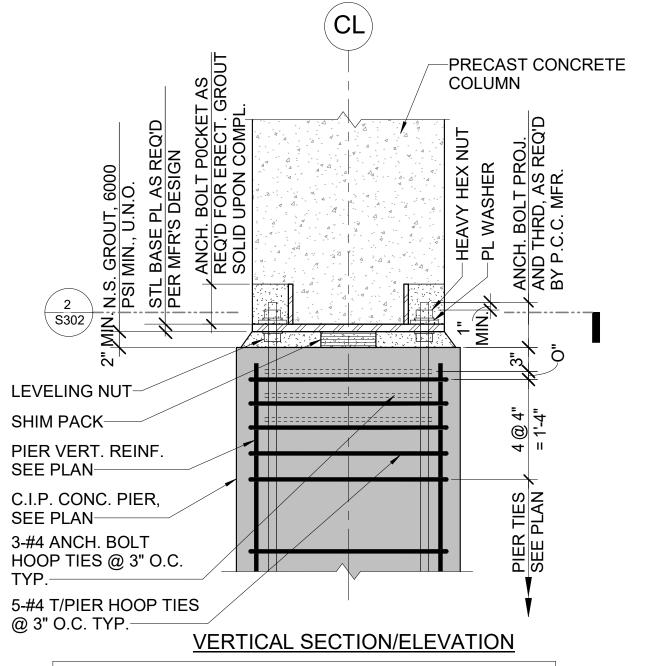
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North Stair Enclosure Enlarged Plan

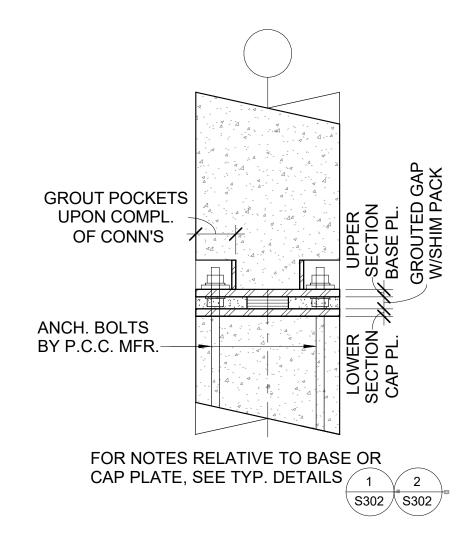


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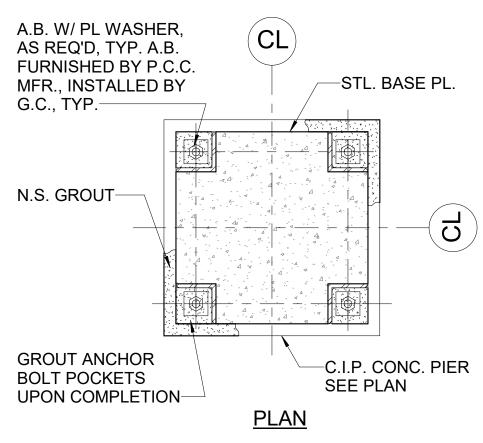


COLUMN BASE PLATE NOTES: FOOTPRINT OF BASE PLATE SHALL AT A MINIMUM MATCH OVERALL DIMENSIONS OF COLUMN CROSS-SECTION. PARTIAL PLATES COVERING PORTION OF CROSS-SECTION ARE NOT PERMITTED. EACH INDIVIDUAL BASE PLATE SHALL BE FABRICATED FROM ONE (1) PIECE OF STEEL PLATE. WELDED PLATES ARE NOT PERMITTED.

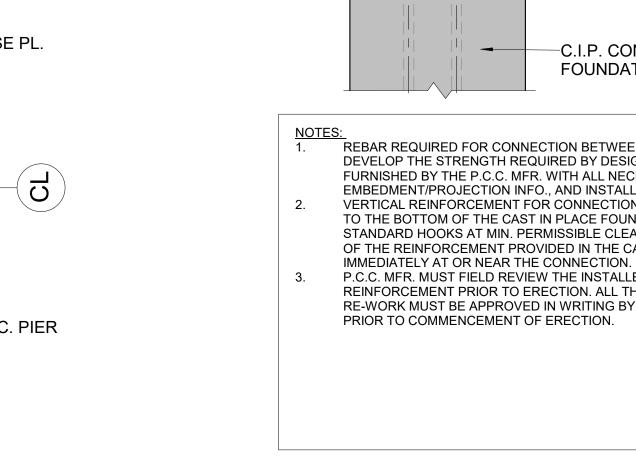
TYP. COLUMN-TO-PIER OR FOOTING CONNECTION DETAIL S302 SCALE: N.T.S.



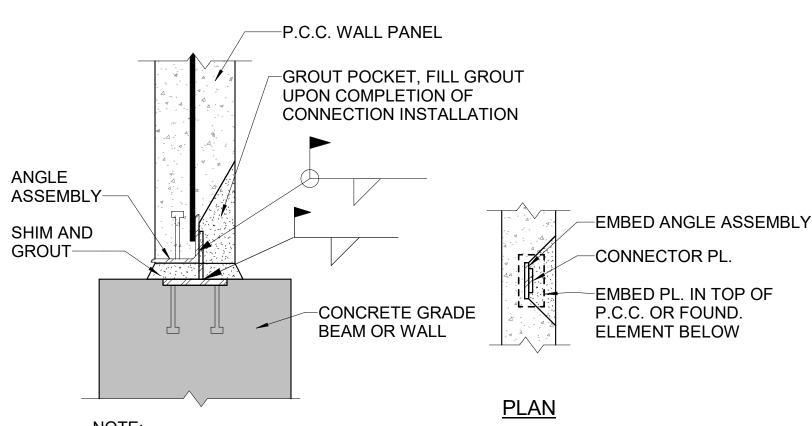
TYP. P.C.C. COLUMN SPLICE DETAIL S302 SCALE: N.T.S.



TYP. P.C.C. COLUMN BASE PLATE DETAIL S302 SCALE: N.T.S.

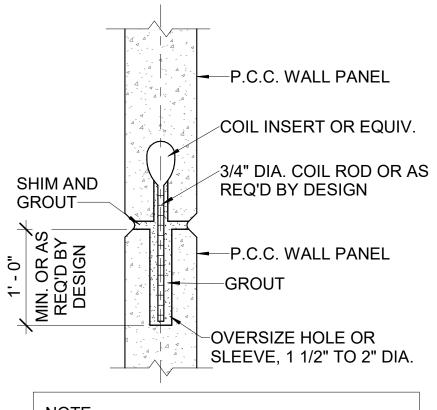


TYP. P.C.C. SHEAR WALL PANEL CONNECTION DETAIL S302 SCALE: N.T.S.



THIS DETAIL MAY BE USED FOR CONNECTIONS BETWEEN THE FOUNDATION AND P.C.C. WALL PANEL AS WELL AS BETWEEN P.C.C. WALL PANELS.

6 TYP. P.C.C. WALL PANEL INTERCONNECTION DETAIL S302 SCALE: N.T.S.



PRECAST CONCRETE WALL

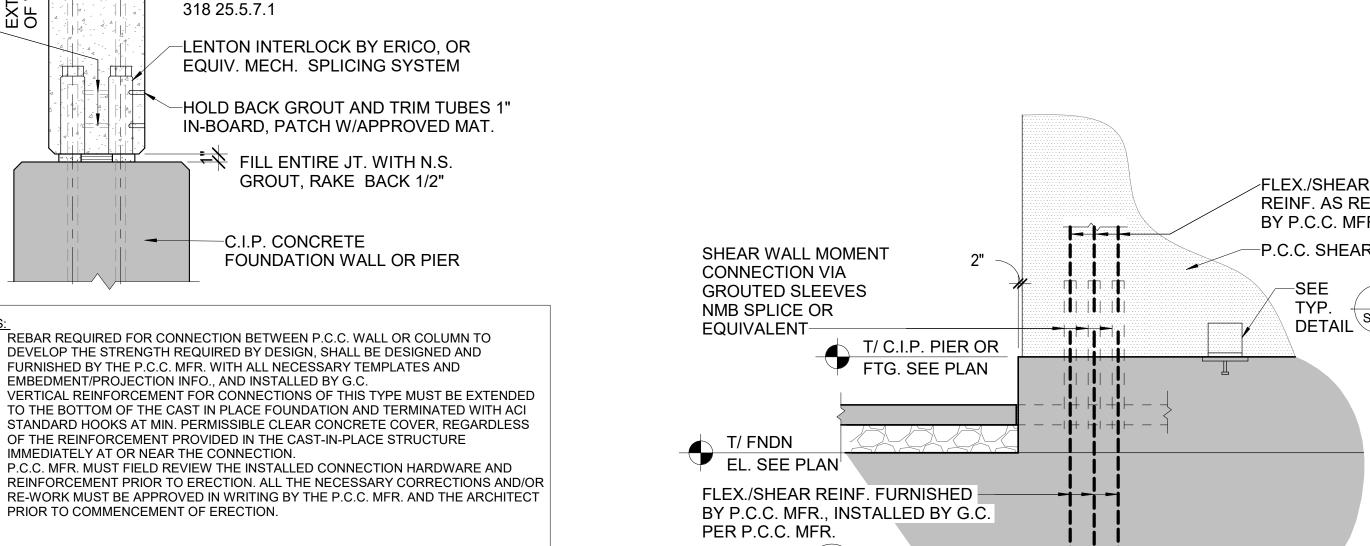
DESIGN, TO ACCEPT SPLICE PRODUCT CONFORMING TO ACI

-THRD. BAR, SIZE BASED ON MFR'S

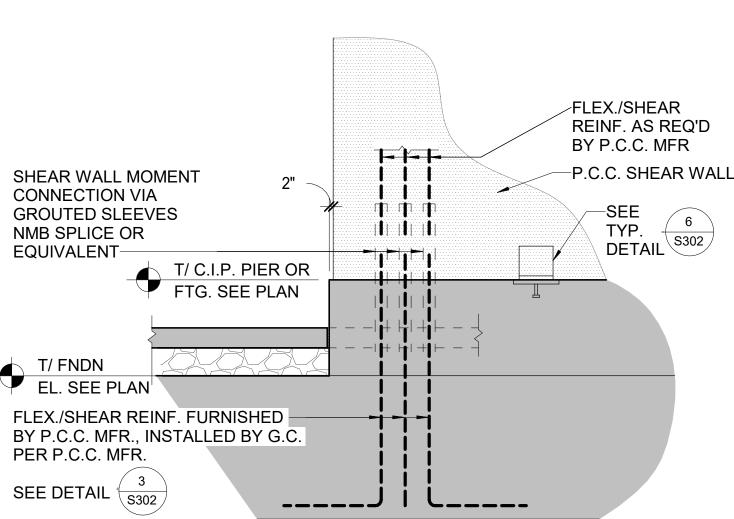
PANEL

SHIM AS REQ'D BETW. CONNECTIONS SPACED AT 5'-0" O.C. OR AS REQ'D BY DESIGN. FULLY GROUT HORIZ. GAP BETW. CONNECTIONS.

TYP. P.C.C. WALL PANEL 7 INTERCONNECTION DETAIL S302 SCALE: N.T.S.



4 TYP. SECTION AT THE END OF SHEAR WALL S302 SCALE: N.T.S.



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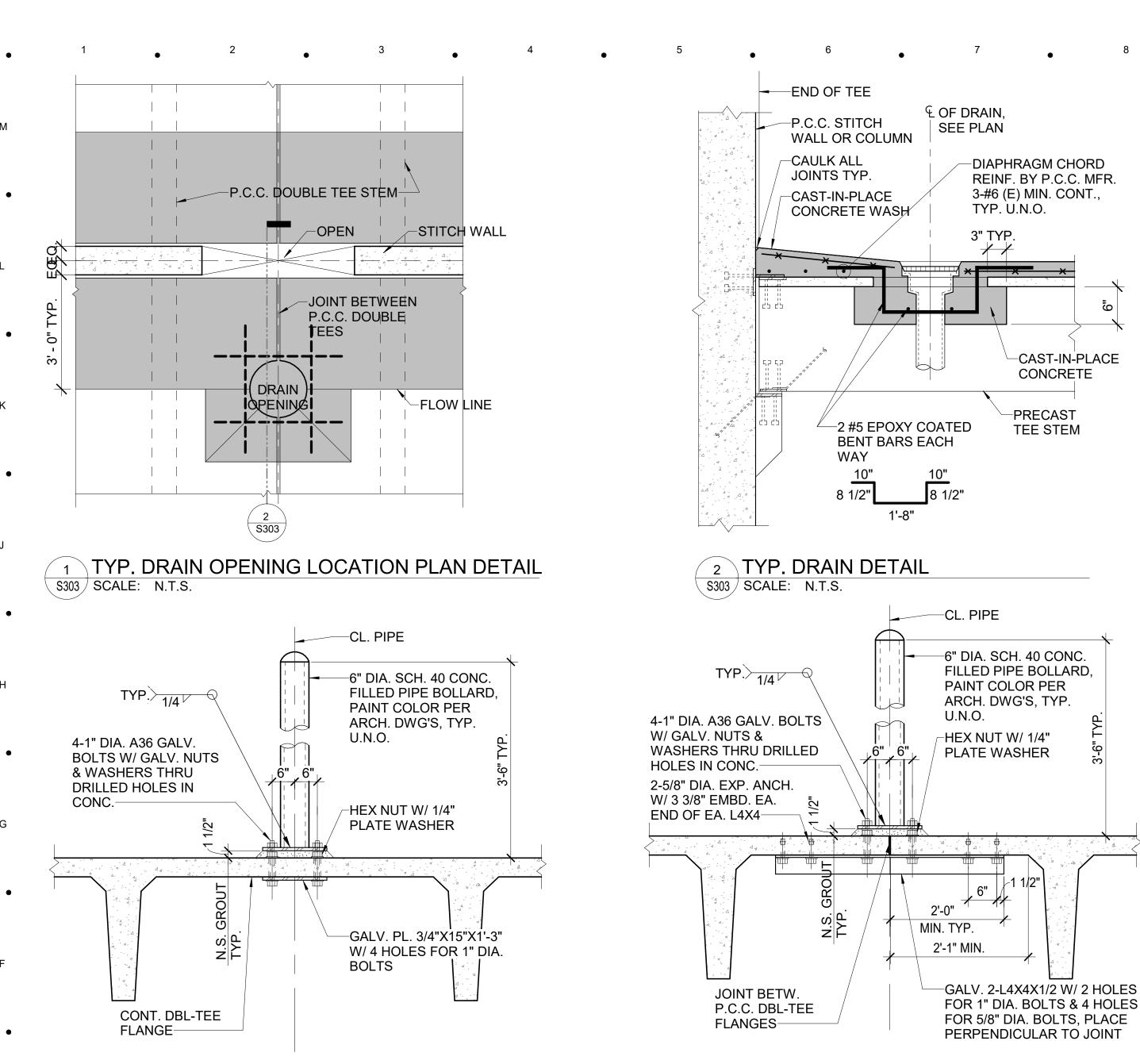
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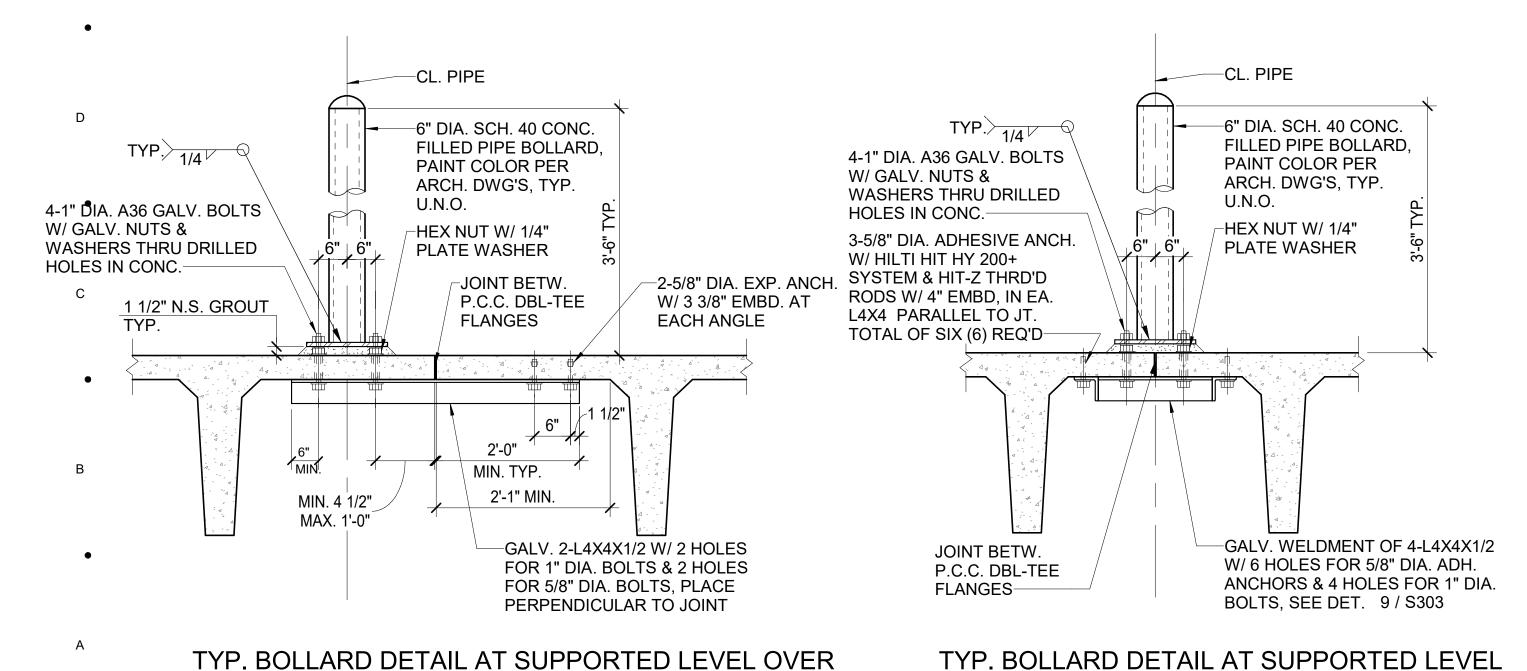
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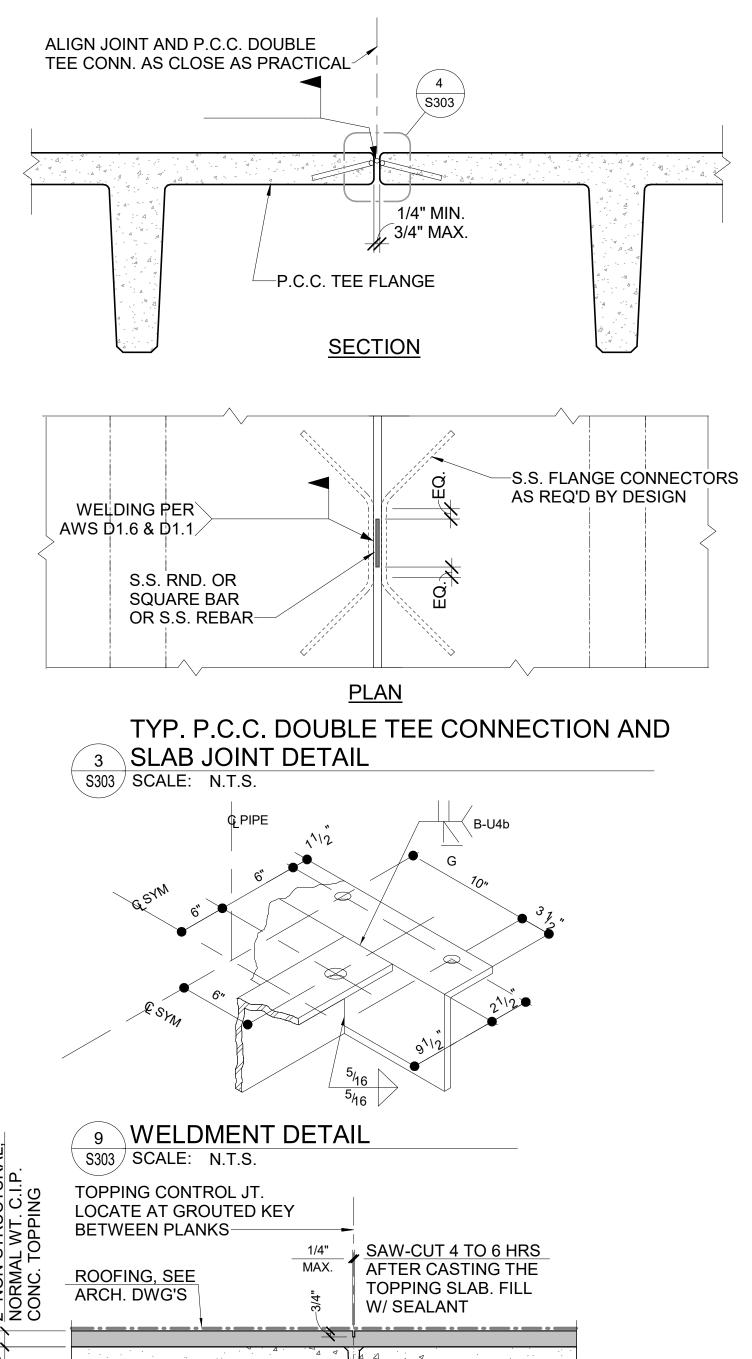
TYP. BOLLARD DETAIL AT SUPPORTED LEVEL

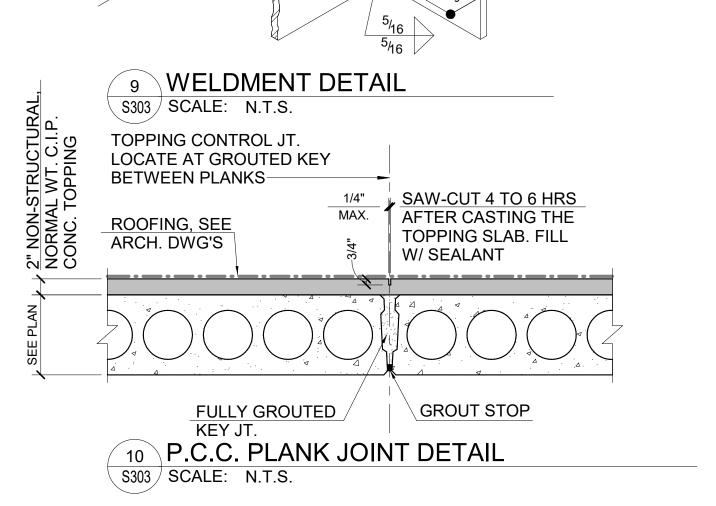
DISCONT. FLANGE BETW. STEMS

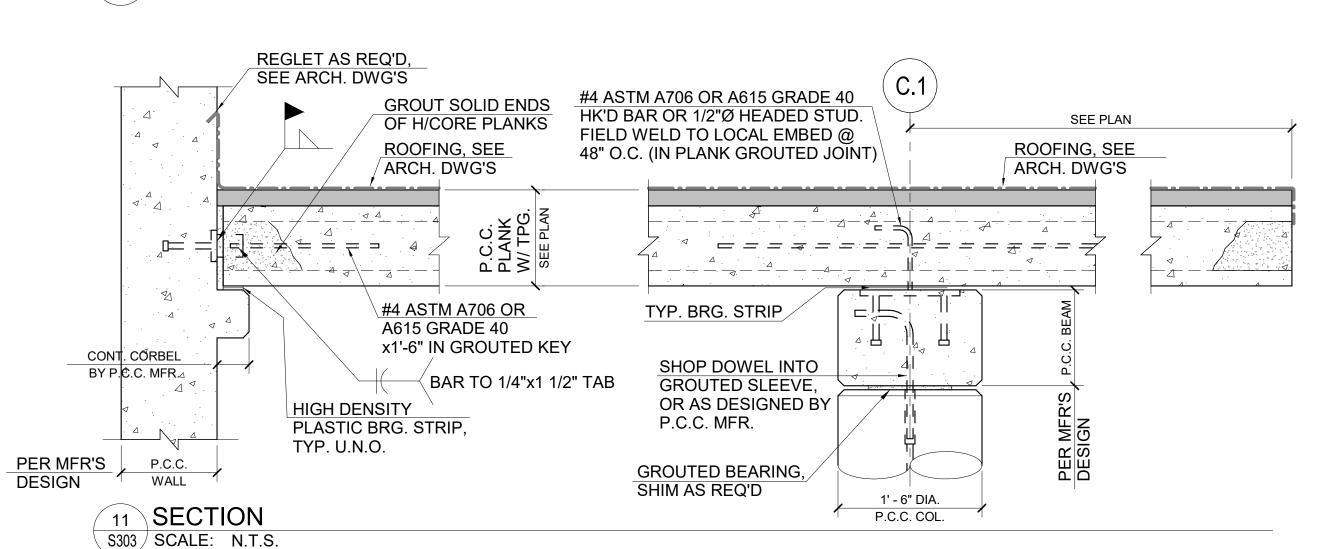
S303 SCALE: N.T.S.

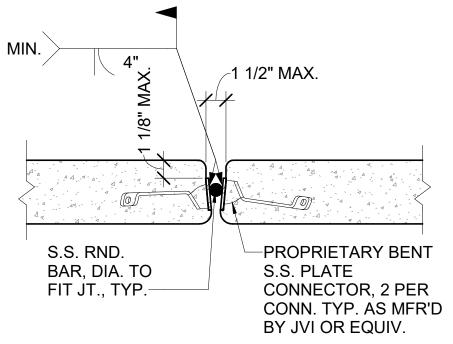
S303 SCALE: N.T.S.

TYP. BOLLARD DETAIL AT SUPPORTED LEVEL 8 CENTERED ON JOINT BETW. TRIMMED FLANGES S303 SCALE: N.T.S.

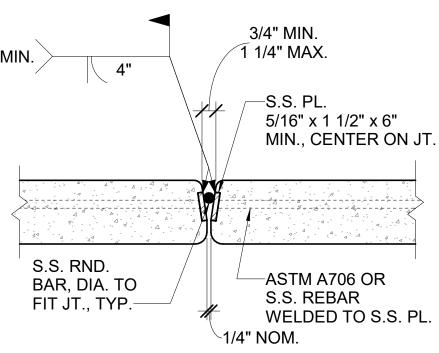




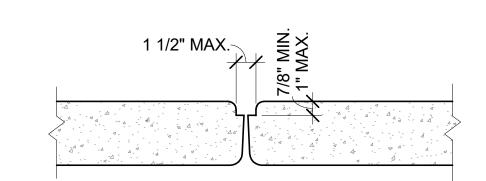




AT CONNECTOR OPTION 1



AT CONNECTOR OPTION 2



BETWEEN CONNECTIONS

NOTE: FLANGE CONNECTOR SPACING SHALL BE ESTABLISHED BY THE P.C.C.. MFR. BASED ON PCI RECOMMENDATIONS, BUT SHALL NOT EXCEED 5'-0" O.C.

4 TYP. CONNECTION DETAILS S303 / SCALE: N.T.S.

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Project Issued:

Schematic Design

Sheet Issued: 09/23/2022

11-22107.00-1

100 SE 3rd Avenue, 10th Floor

Fort Lauderdale, Florida 33394

Owner: Owner

Naples, Florida 34102

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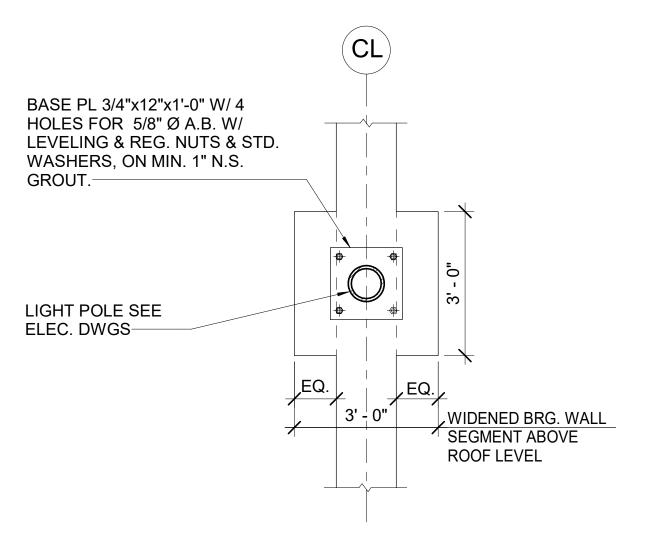
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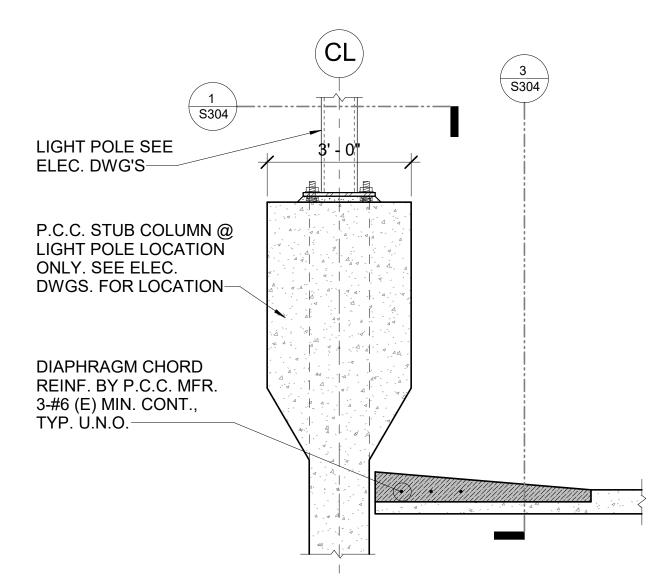
Sheet No.

S303

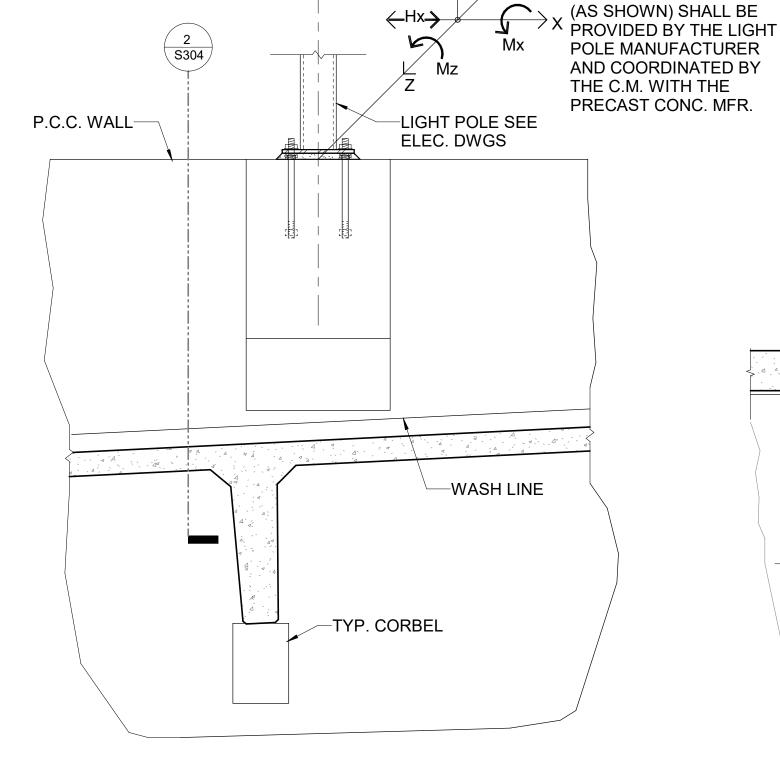
Autodesk Docs://2204.01 - Naples Playhouse Parking Garage/1st_Ave_Naples Playhouse_S_Central_R22.rvt 12/29/2022 11:33:32 AM



1 TYP. LIGHT POLE SUPPORT PLAN SCALE: N.T.S.



2 TYP. LIGHT POLE SUPPORT SECTION SCALE: N.T.S.

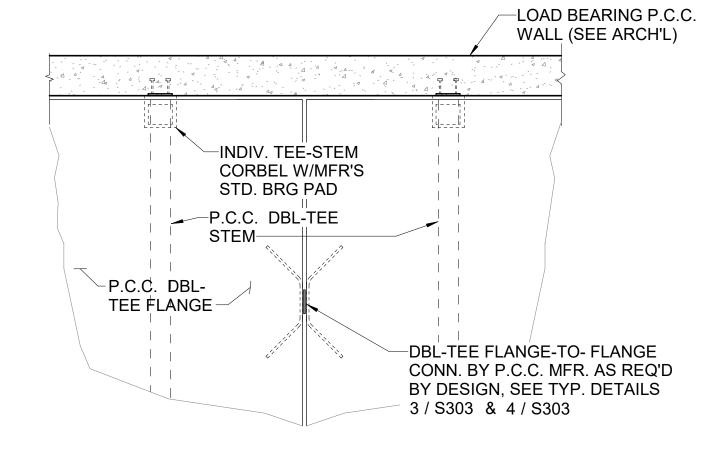


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FORCES FOR THE LIGHT

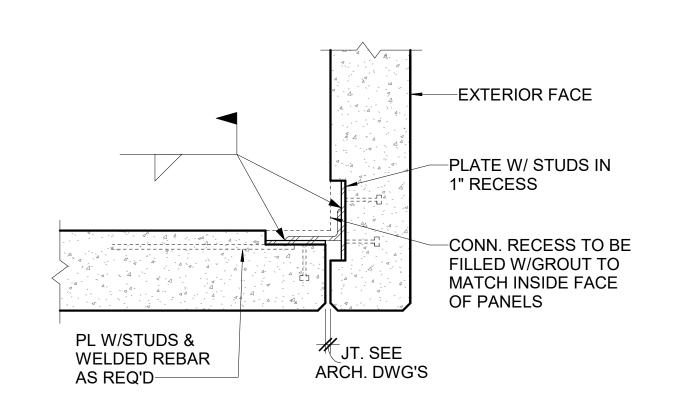
POLE SUPPORT DESIGN

3 TYP. LIGHT POLE SUPPORT ELEVATION S304 SCALE: N.T.S.



• 8 • 9 • 10 •

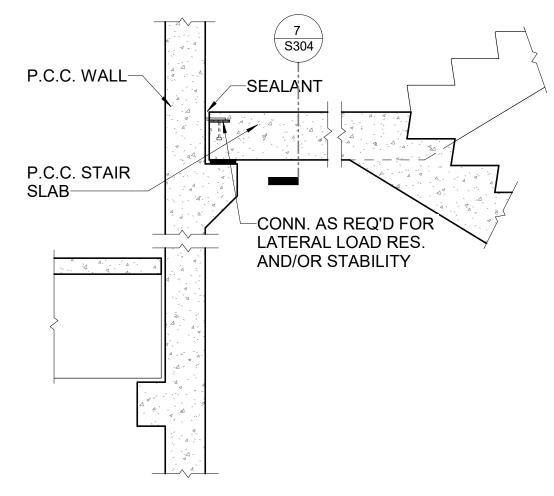
4 S303 - TYP. PLAN DETAIL AT EXTERIOR P.C.C. WALL S304 SCALE: N.T.S.



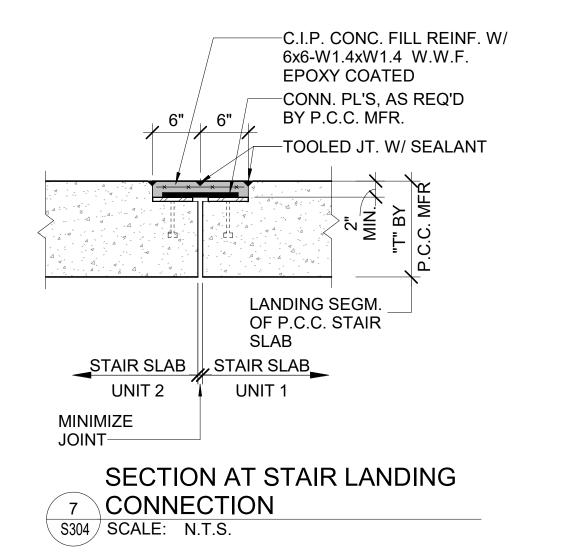
P.C.C. WALL TO P.C.C. WALL CORNER

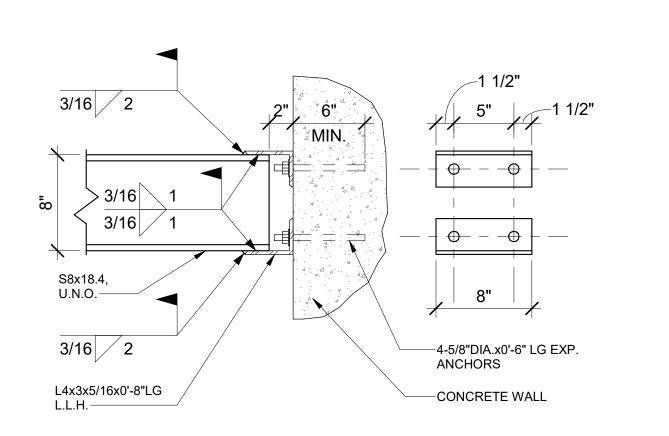
5 CONNECTION PLAN DETAIL

\$304 SCALE: N.T.S.

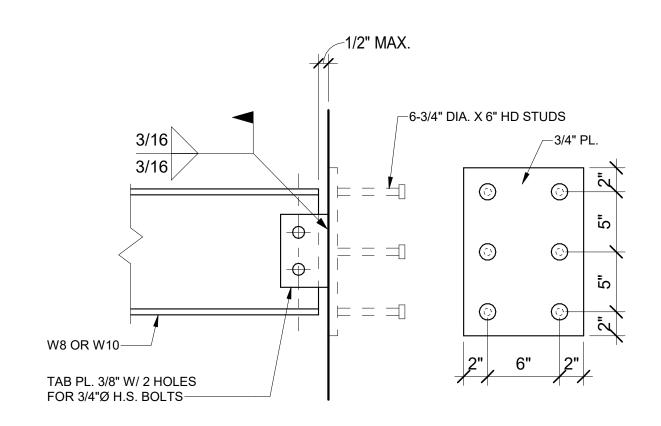


6 SECTION AT STAIR
S304 SCALE: N.T.S.





8 DETAIL - ELEVATOR DIVIDER BEAM @ P.C.C. WALL S304 SCALE: N.T.S.



9 SCALE: N.T.S.

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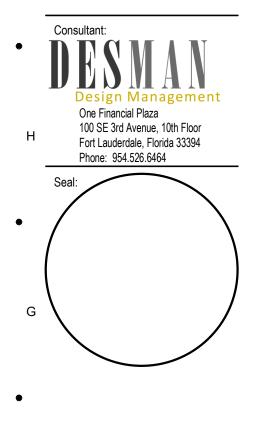
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Phone: 239-643-3103 Fax: 239-643-7435

1st Avenue S.
Public Parking
Garage

Owner:
Owner



Project Phase:
Schematic Design

Project #: 11-22107.00-1
Project Issued: 09/23/2022

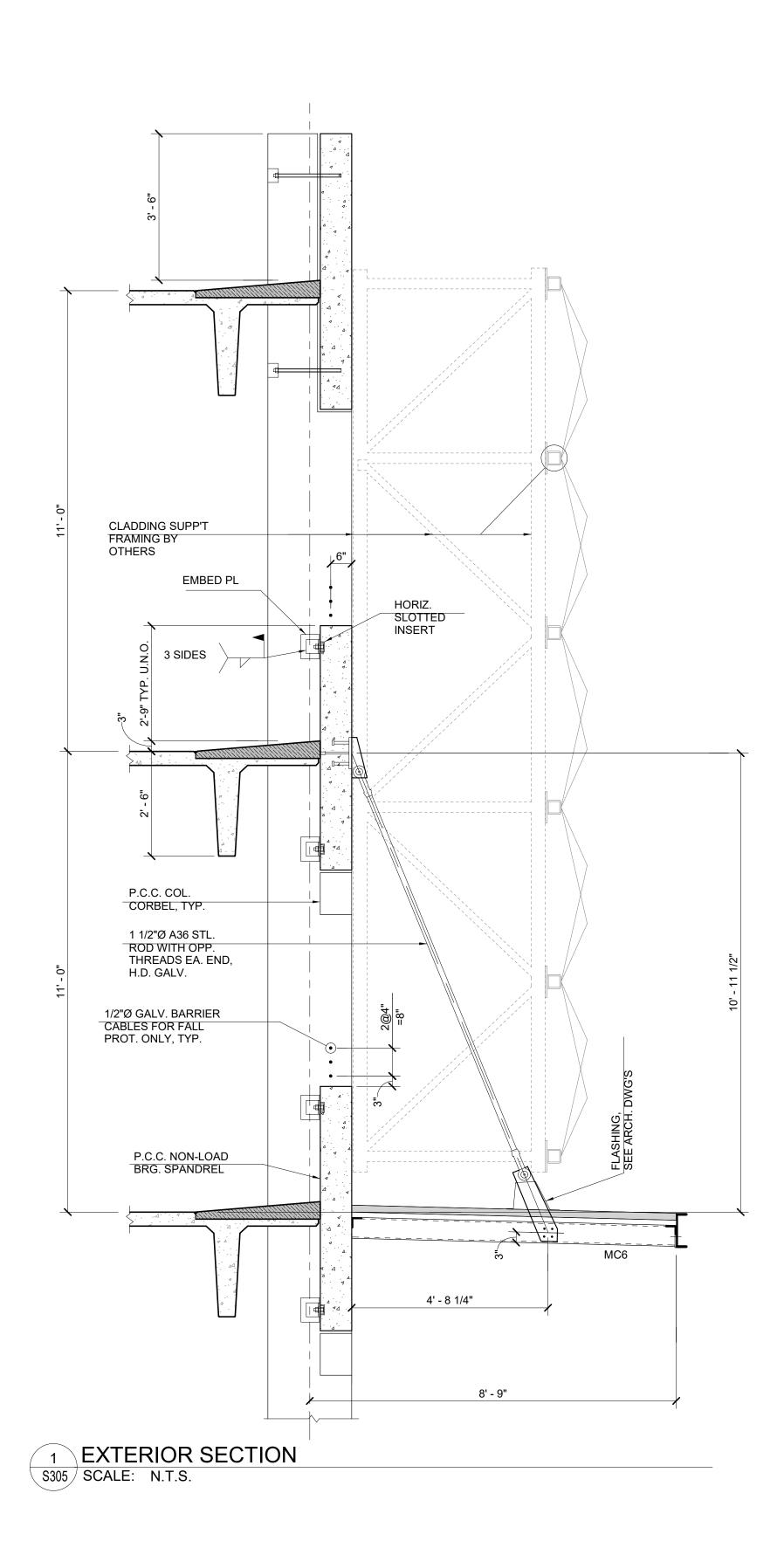
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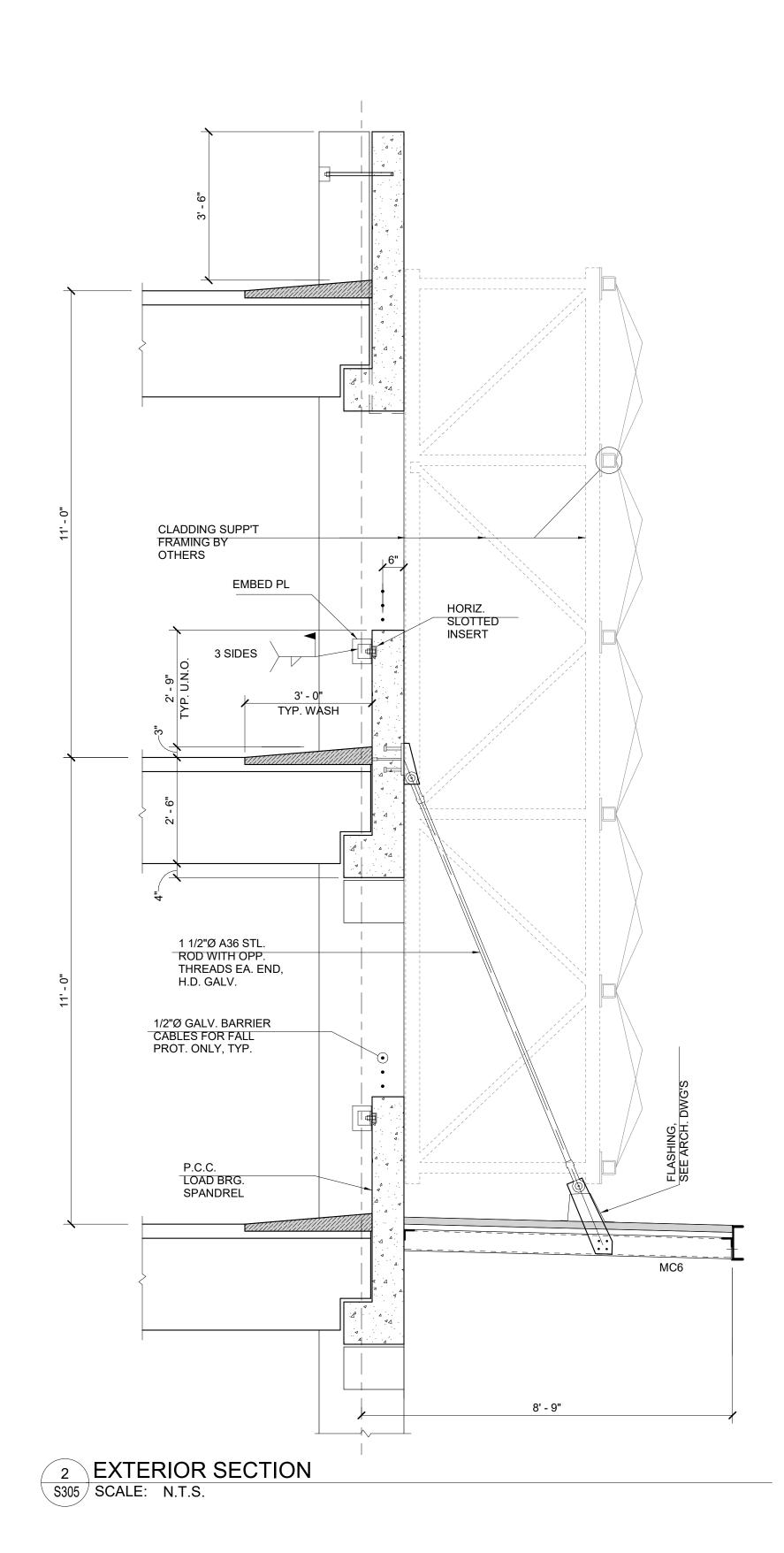
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Details

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. 1 . 2 . 3 . 4 . 5 . 6 . 7 . 8 . 9 . 10 . 11 . 12



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Orainat:

1st Avenue S. Public Parking Garage

Owner:
Owner

H

Design Management
One Financial Plaza
100 SE 3rd Avenue, 10th Floor
Fort Lauderdale, Florida 33394
Phone: 954.526.6464

Seal:

Schematic Design

Project #: 11-22107.00-1
Project Issued:
Sheet Issued: 11/08/22

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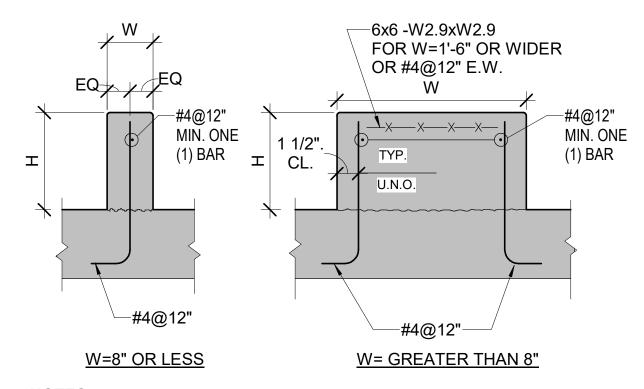
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Precast Sections
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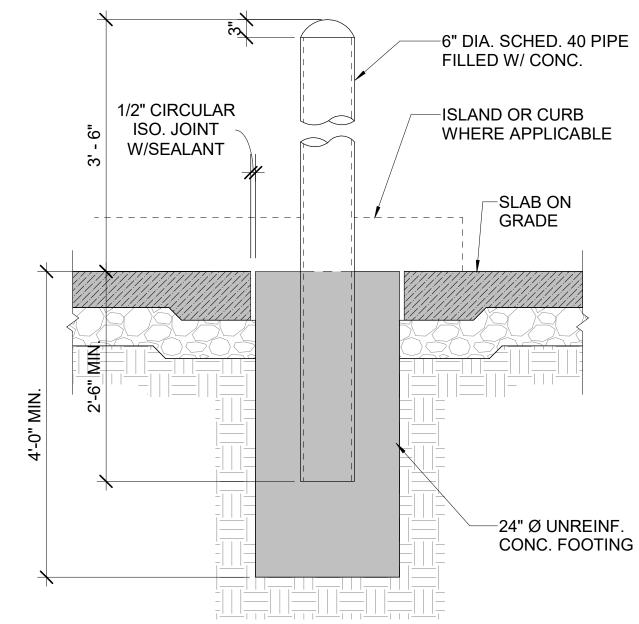
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PROVIDE CONTINUOUS SEALANT ALONG ALL SLAB TO CURB JOINTS. 2. FOR CURB W AND H DIMENSIONS, SEE ARCH. DWGS.

1 CONCRETE CURB DETAIL

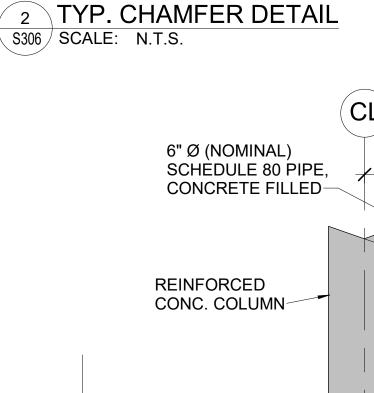
S306 SCALE: N.T.S.



NOTE: PROVIDE CONTINUOUS SEALANT ALONG SLAB TO BOLLARD JOINT.

SLAB BOLLARD DETAIL IN SLAB-ON-GRADE OR PAVEMENT

S306 / SCALE: N.T.S.



EXTERIOR FACE

NOTE: PROVIDE CHAMFER THUS @ ALL EXTERIOR CORNERS, TYP. U.N.O.

SLAB EDGE-SLAB SOFFIT

PROVIDE DRIP THUS @ ALL SUPPORTED SLAB EDGES, U.N.O. 3 TYP. "DRIP" DETAIL

S306 SCALE: N.T.S.

(CL)SEE ARCH. -SONOTUBE MIN. 2'-0"Ø DWG'S. FILL WITH CONCRETE AFTER INSTALLATION OF BOLLARD REMOVE FORM PRIOR TO S.O.G. PLACEMENT. -1" THICK x 12"x1'-0" BASE PLATE ON 1" NON-SHRINK GROUT WITH ANCHOR BOLT HOLE ON 9 x 9 SQ. PATTERN CENTERED ON BOLLARD. T/ S.O.G. EL. -PROVIDE STD. HOOK WHERE STRAIGHT **EXTENSION IS NOT** 2-#5 2' - 6" EA.-WAY-AVAILABLE FOR PILE CAP DIMENSIONS, SEE SCHEDULE -TOTAL 4-3/4"Ø A307 ANCHOR BOLTS, 1'-6" MIN. PIPE BOLLARD AT FOUNDATION INTO COLUMN FOOTING 8 DETAIL

> AT ALL INTERRUPTED BARS. PLACE HK'S IN VERT. PLANE-CORNERS) OPENING 2" CLR TO FREE EDGE OR OBSTRUCTION SEE SCHED 4'-0" MAX ACI STD ÓN DWG. SOÓ2 DEV. LENGTH FOR HK'D BAR USE LAP CLASS A,

> > WHICH HAS NOT BEEN SPECIFICALLY DETAILED. WHEN SUCH DETAILS ARE PROVIDED ON THE STRUCTURAL DWG'S, REINF. SHOWN MUST BE

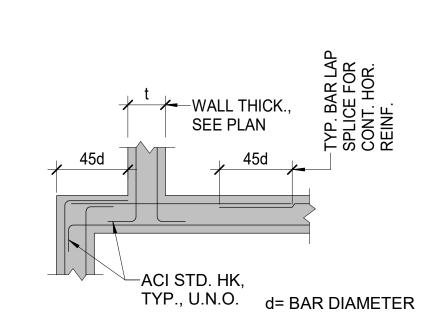
S306 SCALE: N.T.S.

-6" DIA. (NOM.) SCHED. 40 EMBED PL 3/4"x12"x1'-0" PIPE CONC. FILLED W/ CONC. W/ 4-3/4"øx9" H.D. WELDED STUDS ON SQUARE FOR DIM'S SEE DETAIL PATTERN @ 8" O.C.-8 THIS DWG. -EDGE OF ISLAND 28-#5 (7 BARS WHERE APPLICABLE T&B EA. WAY)--SLAB ON GRADE W/ STRUCTURAL FIBER TYP. 3' - 0" 3' - 0"

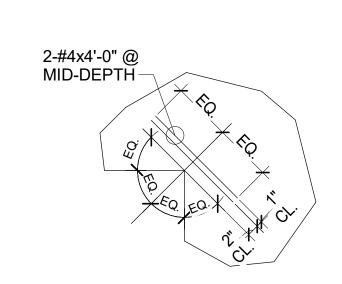
NOTE: NO SLAB CONTROL OR CONSTRUCTION JOINTS SHALL BE LOCATED WITHIN 5'-0" FROM BOLLARD CENTERLINE

ALTERNATE PIPE-BOLLARD DETAIL 9 ON SLAB-ON-GRADE S306 SCALE: N.T.S.

PASSENGER VEHICLE IMPACT

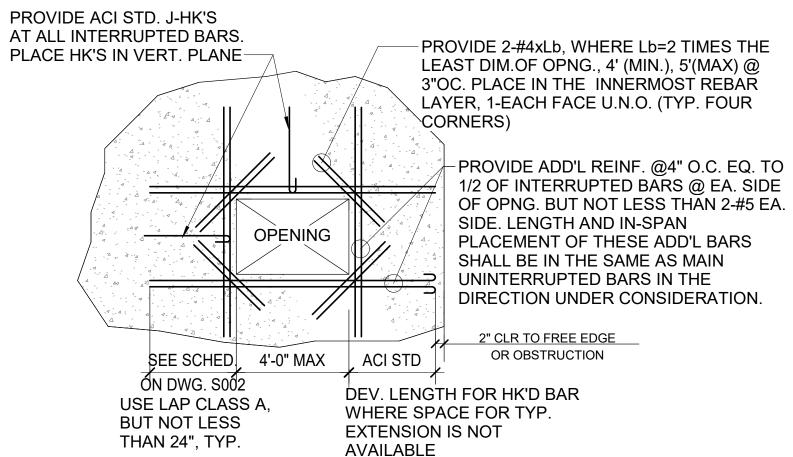


BAR SPLICES IN WALL DOUBLE 10 LAYER TYP. 、S306 / SCALE: N.T.S.



S306 SCALE: N.T.S.

RE-ENTRANT CORNER BARS TYP. 11 DETAIL S306 SCALE: N.T.S.



THIS DETAIL APPLIES TO SLAB AND WALL OPNG'S, REINFORCEMENT FOR SUPPLEMENTED WITH DIAGONAL CORNER REINF. SHOWN IN THIS DETAIL.

12 MIN. REINFORCEMENT AT SLAB OR WALL OPENINGS

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1st Avenue S. **Public Parking** Garage

Owner

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> Project Phase: Schematic Design

Project #: 11-22107.00-1 Project Issued:

Sheet Issued: 09/23/2022

Date 30% SD 09.23.22 O1.03.23 60% DD

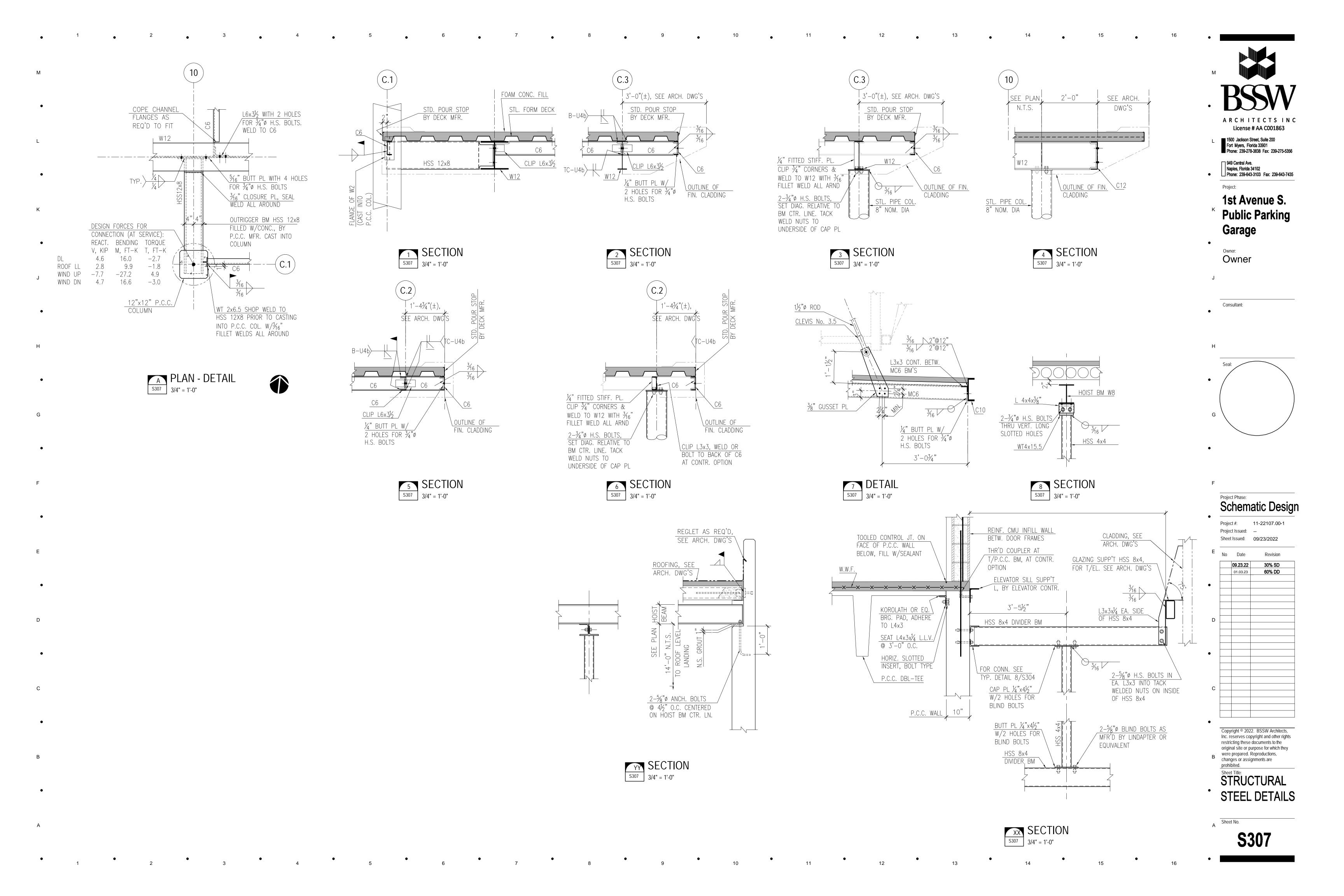
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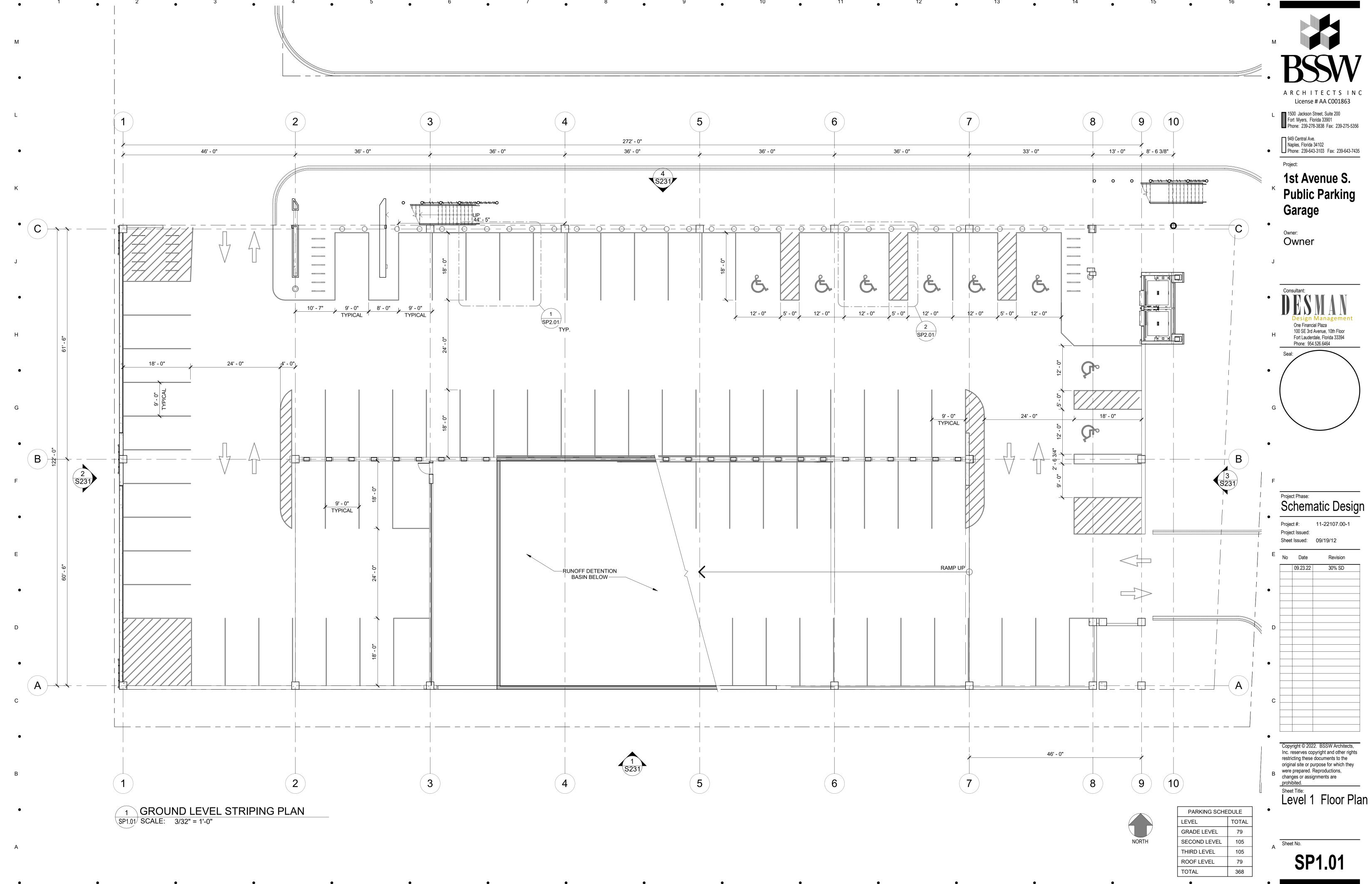
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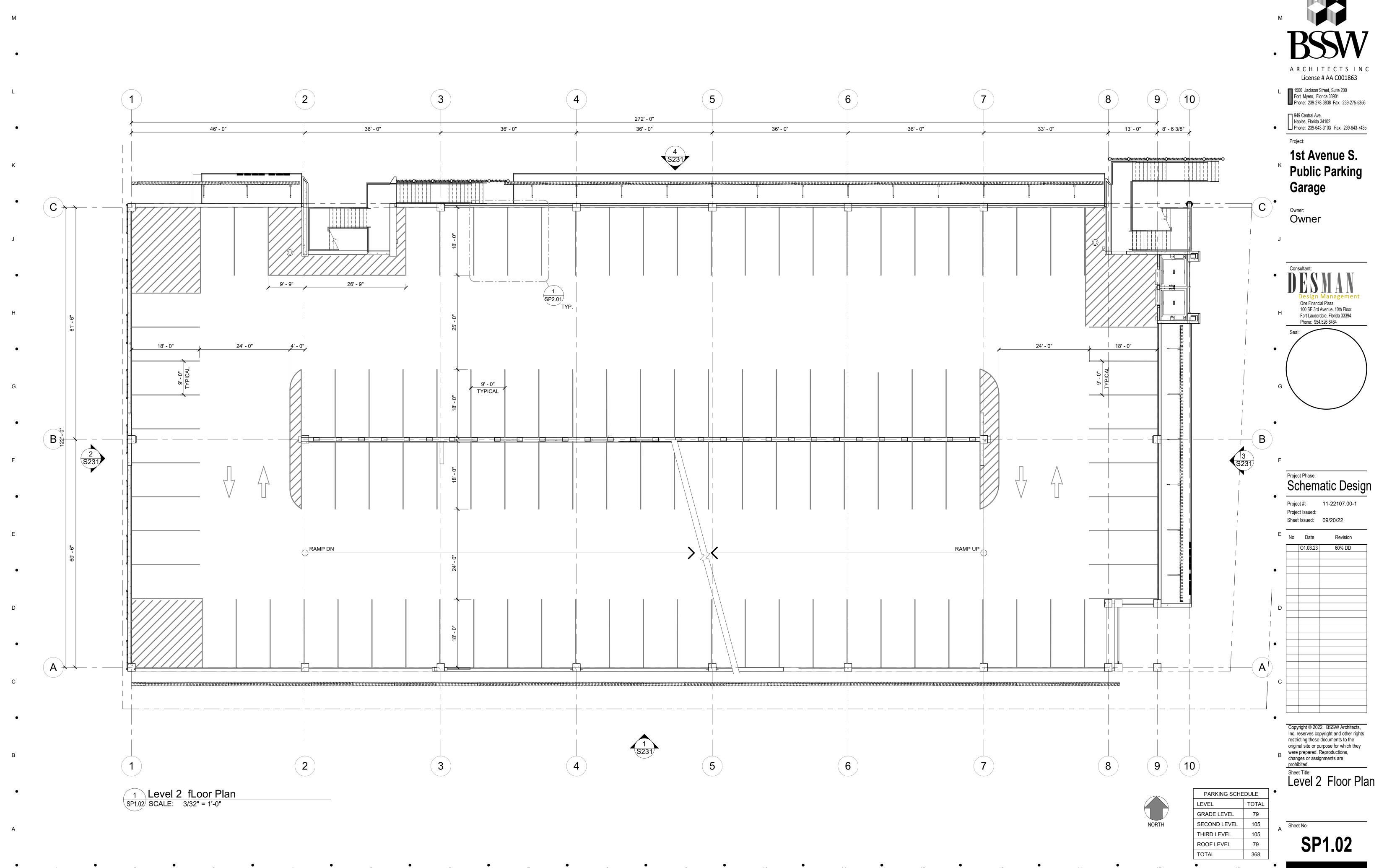
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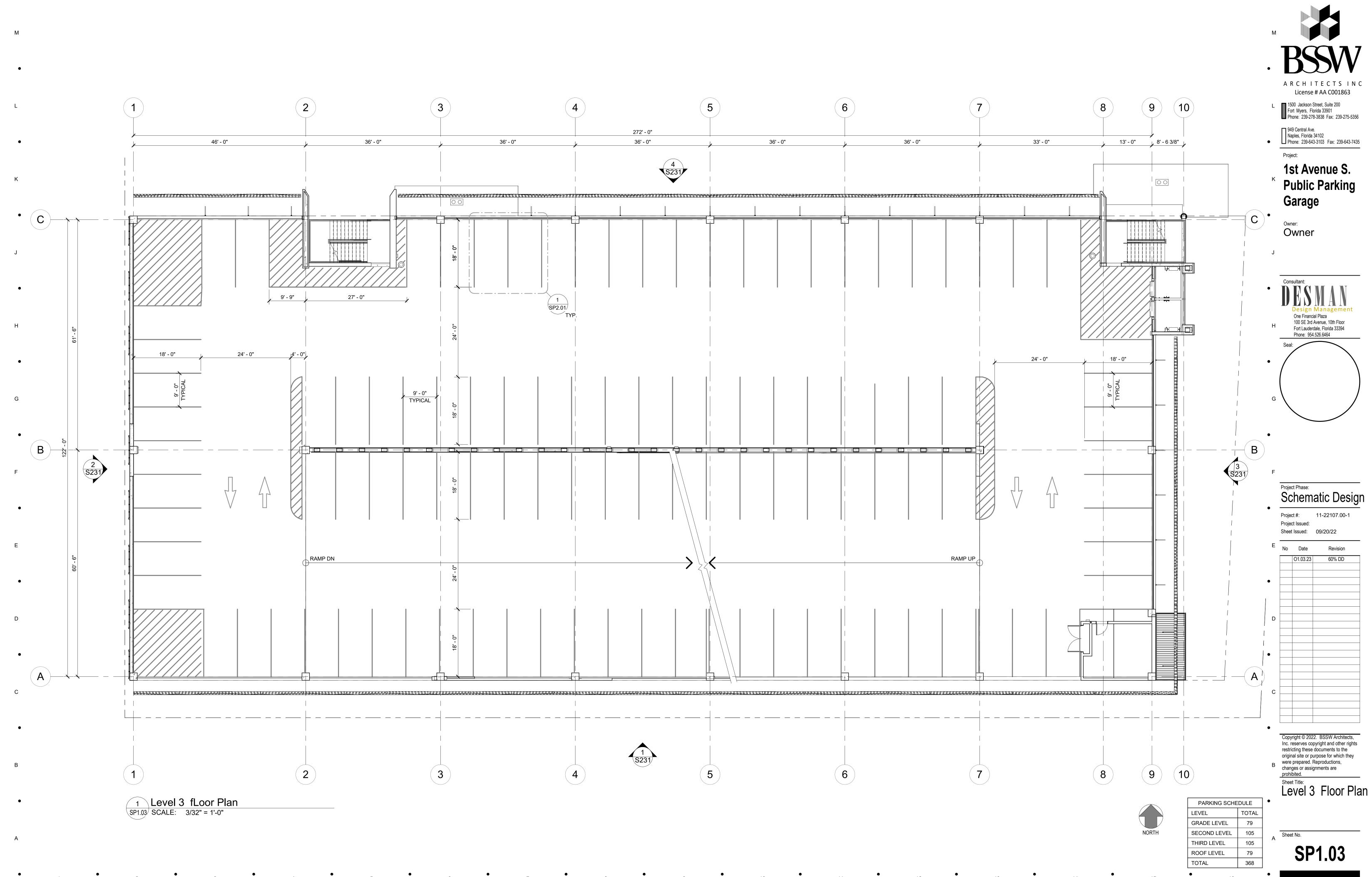
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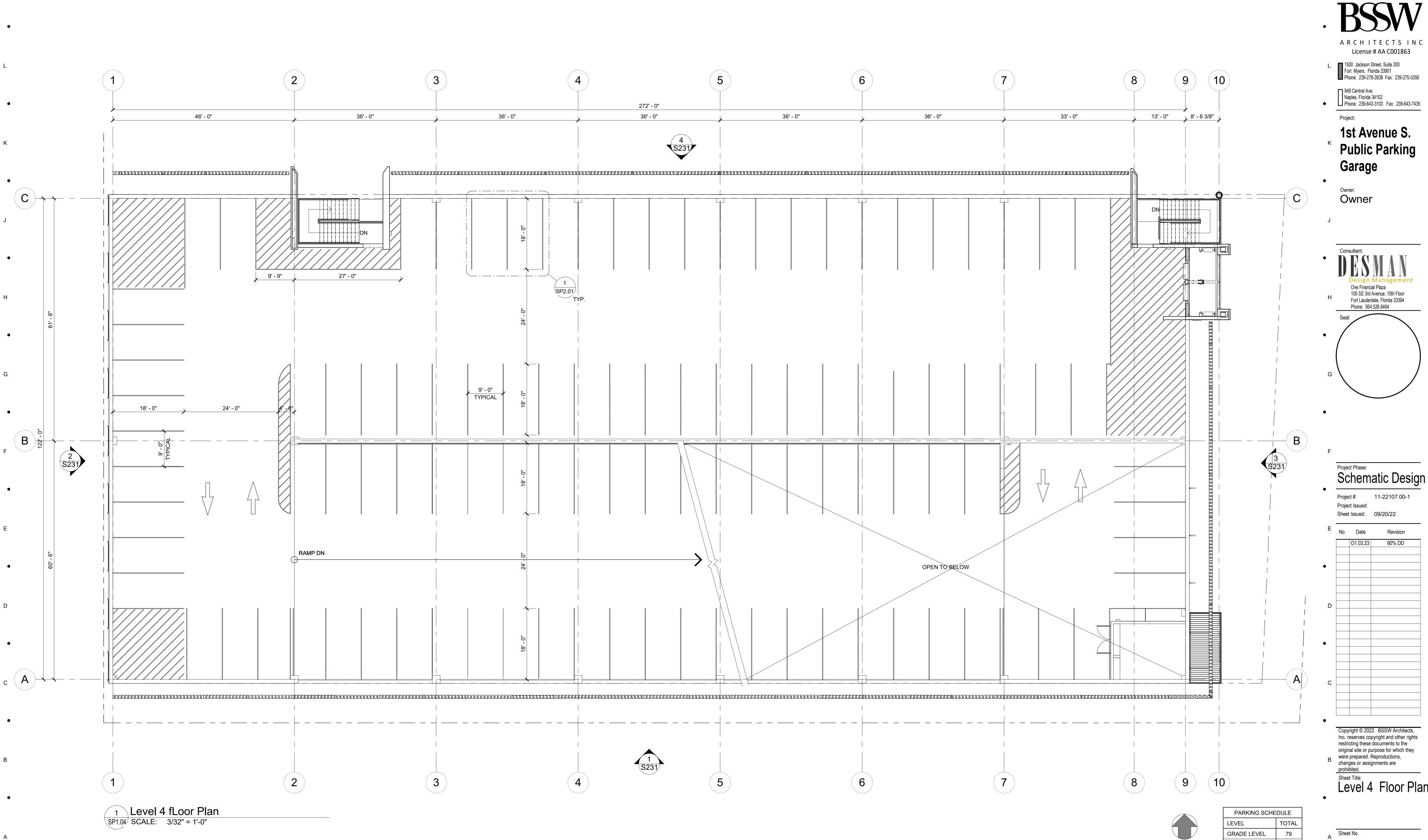




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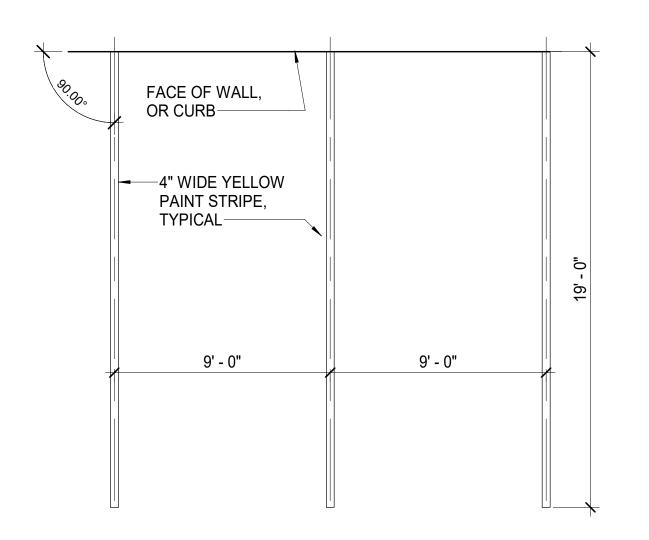
Level 4 Floor Plan

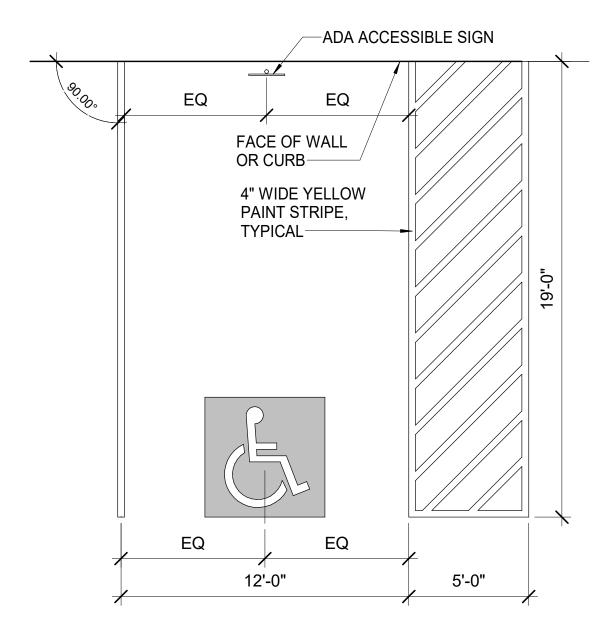
105

SECOND LEVEL

THIRD LEVEL **ROOF LEVEL**

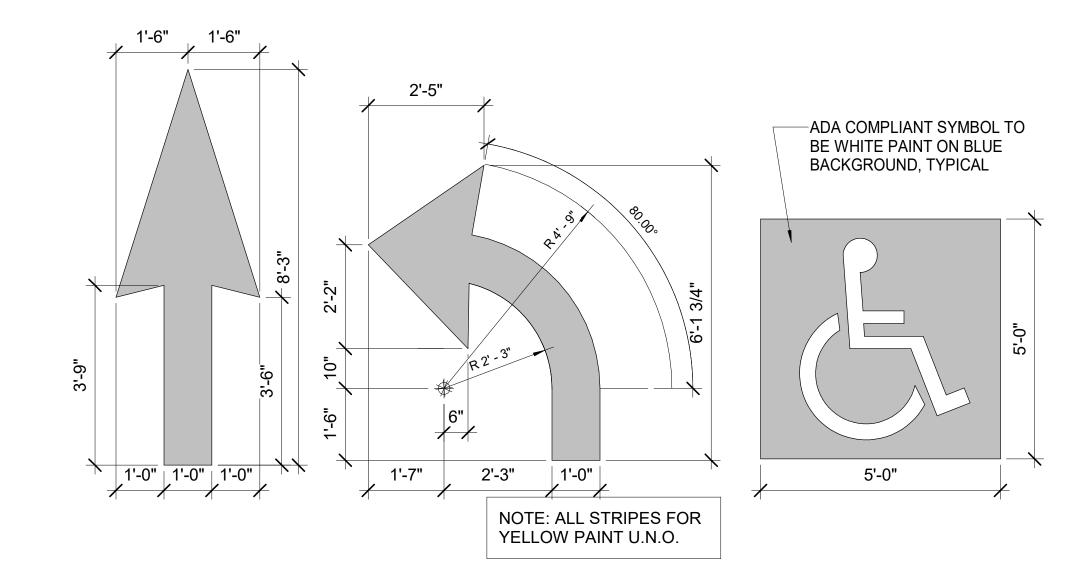
TOTAL





1 90 DEGREE STALL TYPICAL STRIPING DETAILS SP2.01 SCALE: 1/4" = 1'-0"

2 ADA STALL TYPICAL STRIPING DETAIL SP2.01 SCALE: 1/4" = 1'-0"



4 TYPICAL STRIPING DETAILS SP2.01 SCALE: 1/2" = 1'-0"

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Project Phase:

Project #: 11-22107.00-1

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SP2.01